Exhibit 3.7

FROELICH ENGINEERS

(Preliminary) Stormwater Drainage Report

CATT INTENSIVE SERVICES BUILDING (ISB)

17911 NW Evergreen Place
Beaverton, Oregon 97006

For

Washington County Support Services

NOT FOR CONSTRUCTION

RENEWS: 12/31/23

Regular maintenance and inspection are required on all components of the stormwater system. This plan provides instruction on how to maintain and inspect the system.

Prepared by: Evan Eykelbosch, PE and Ben Ullmann, PE
Froelich Engineers
17700 SW Upper Boones Ferry Rd, Suite 115
Portland, OR 97224
Froelich Project Number: 22-C023
Date: August 31, 2023

Designer's Certification and Statement

"I hereby certify that this Stormwater Management Report for the CATT Intensive Services Building (ISB) project has been prepared by me or under my supervision and meets the minimum standards of the City of Beaverton and normal standards of engineering practice. I hereby acknowledge and agree that the jurisdiction does not and will not assume liability for the sufficiency, suitability, or performance of drainage facilities design by me."

Evan Eykelbosch, PE

Contents

I. Project Overview and Description

This stormwater report has been prepared in accordance with the City of Beaverton requirements and the Clean Water Services Design and Construction Standards (CWS) to support the permit applications for the proposed project improvements.

The CATT Intensive Services Building (ISB) is a proposed remodel of and addition to the existing building located at 17911 NW Evergreen Place (see Appendix A: Vicinity Map). Other site improvements include reconstruction of the ADA parking stalls, ramps, entryways, and sidewalks to be ADA compliant.

Existing Conditions

The existing site consists of one tax lot with an existing building, sidewalks, and parking lots along the south side of the building. The topography is slightly sloping and drains to the south towards the private street. Roof runoff is collected in roof drains that ultimately convey the water to the public stormwater system. Parking lot runoff is collected in catch basins at the low points and is also conveyed to the public stormwater system. The public stormwater system drains into the Tanasbourne Town Center Regional Water Quality Facility and Mitigation Area in the adjacent tax lot east of the project site per as-builts AB006715 dated 1995. The existing soil is mostly Helvetia silt loam with the southeast corner being Dayton silt loam based on a Soil Resource Report from the NRCS (See Appendix D: Soil Resource Report).

An existing condition Basin Map is provided in Appendix B.

Proposed Conditions

The proposed development is to complete tenant improvements to the existing building and construct a two-story addition to the south of the building. This would reconfigure the western parking lot, remove the middle parking lot and keep a majority of the eastern parking lot fully intact. The existing ADA parking stalls would be reconstructed to meet current ADA regulations. Additionally, proposed frontage improvements will include a new vehicular drop off on the northwest side of the building, a new driveway for the west parking lot, closure of the west and middle driveways, reconstruct the east parking lot driveway, and reconstruction of all sidewalks to be 10’ wide curb tight sidewalks and street trees with grates. The project will also need to reconstruct the ADA curb ramp located in the southwest corner of the site with a single sidewalk ramp and construct a midblock curb ramp to cross NW Evergreen Place.

A proposed condition Basin Map is provided in Appendix B.

‘Table 1: Catchment Basins’ provides the basin characteristics for the various catchment areas under the existing and proposed conditions.

The facility is to be maintained by the owner or owner’s representative. The preparer has designed a system that can be easily maintained by maintenance staff. A copy of the O&M Plan shall be provided to all property owners and tenants.

II. Methodology

The City of Beaverton adopted the 2019 Clean Water Services Design and Construction Standards (CWS) for stormwater management. The CWS Standards require all development and redevelopment that creates and/or modifies 1,000 square feet or more of impervious area to comply with stormwater management requirements of hydromodification and treatment.

Hydromodification:
The proposed development has between 12,000 sf and 80,000 sf of new or modified impervious surface (see Appendix B: Basin Map and Areas), is within the developed area, and has a reach-specific risk level rating of moderate, based on the Hydromodification Map Web Tool on Clean Water Services website, designating the site as a Category 2 project, per CWS Table 4-2. This result requires peak flow matching detention or an infiltration facility. The entire site is currently managed by the Tanasbourne Town Center Regional Water Quality Facility and Mitigation Area located adjacent to the site. This system was approved as part of a large development from 1995 (AB006715). Per the Pre-Application notes a Fee-In-Lieu can be paid for the hydromodification of the site.

Water Quality Treatment:
Per the Pre-Application notes a Fee-In-Lieu can be paid for the water quality requirement of the site.

Conveyance:
The conveyance capacity was calculated for the 25-year storm event (3.90 in/24-hr) per Table 4-4 of the CWS standards. A 6" minimum pipe with a 2% or greater slope is sufficient for the peak flow of the site. For the plaza areas, a 4" minimum pipe with a 2% or greater slope is sufficient (see Appendix E: Conveyance Calculations).

- Required Hydromodification Area = 30,191 sf
- Required Treatment Area = 63,462 sf

III. Analysis

This project was analyzed using the Santa Barbara Urban Hydrograph (SBUH) Method with a NRCS Type 1A rainfall distribution for a 24-hour storm. The system was designed using HydroCAD software. Conveyance calculations are based on the Manning Formula for uniform pipe flow. (See Appendix E: Conveyance Calculations).

Table 1: Catchment Basins

Existing

Basin	Area				Total	
	Pervious		Impervious - Disturbed			
	sf	ac	sf	ac	sf	ac
Existing	13,628	0.31	25,458	0.58	39,086	0.90
Total	13,628	0.31	25,458	0.58	39,086	0.90

Proposed

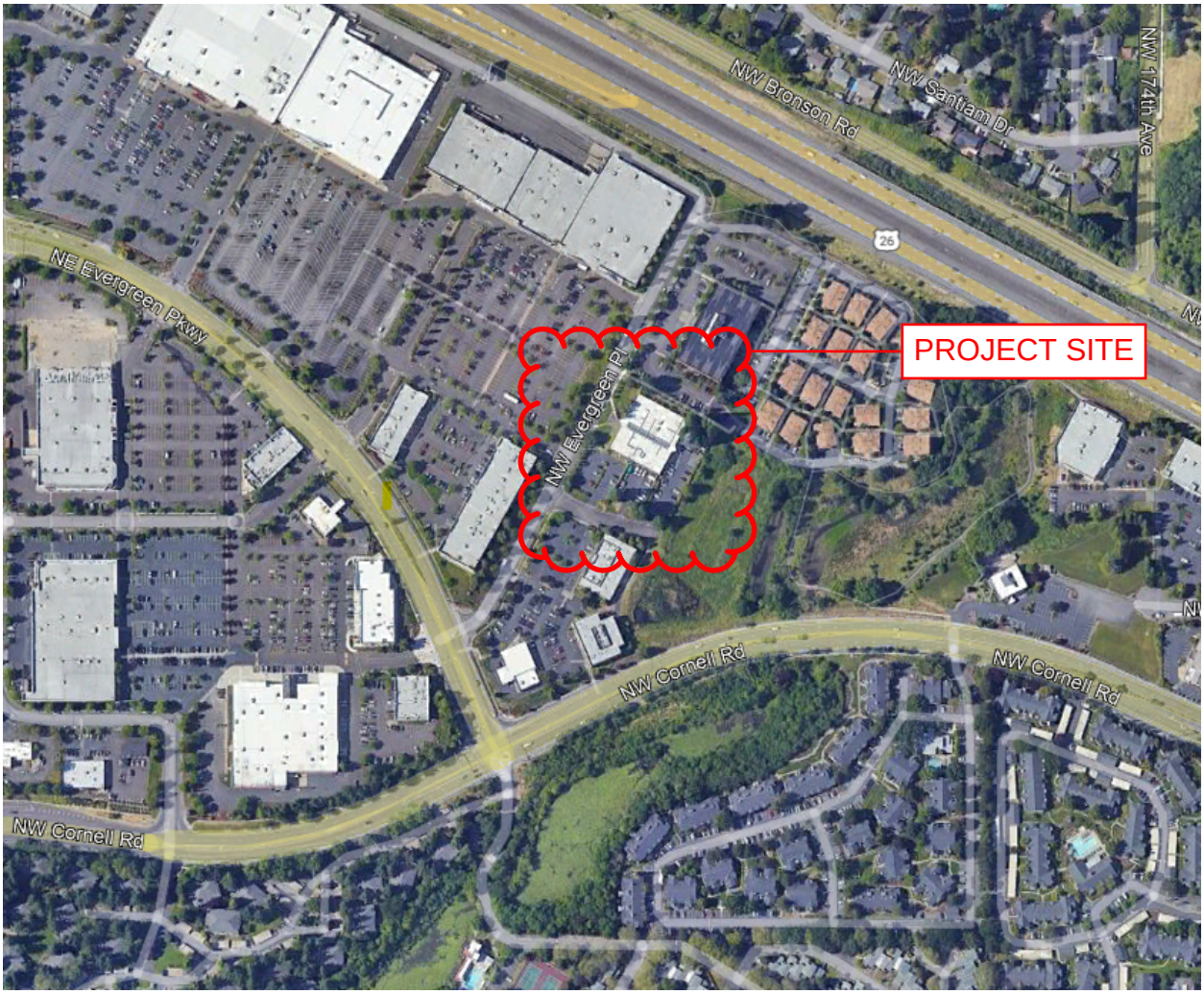
Basin	Area						Total	
	Pervious		Impervious - New		Impervious - Modified			
	sf	ac	sf	ac	sf	ac	sf	ac
Site	8,895	0.20	8,262	0.24	21,929	0.19	39,086	0.90
Total	8,895	0.20	8,262	0.24	21,929	0.19	39,086	0.90

IV. Engineering Conclusion

Based on the requirements of the City of Beaverton and Clean Water Services, all facilities and conveyance components have the capacity to handle the runoff from the required storm events and should be approved as designed.

V. Appendices

Appendix A: Vicinity Map



Appendix B: Basin Map and Areas

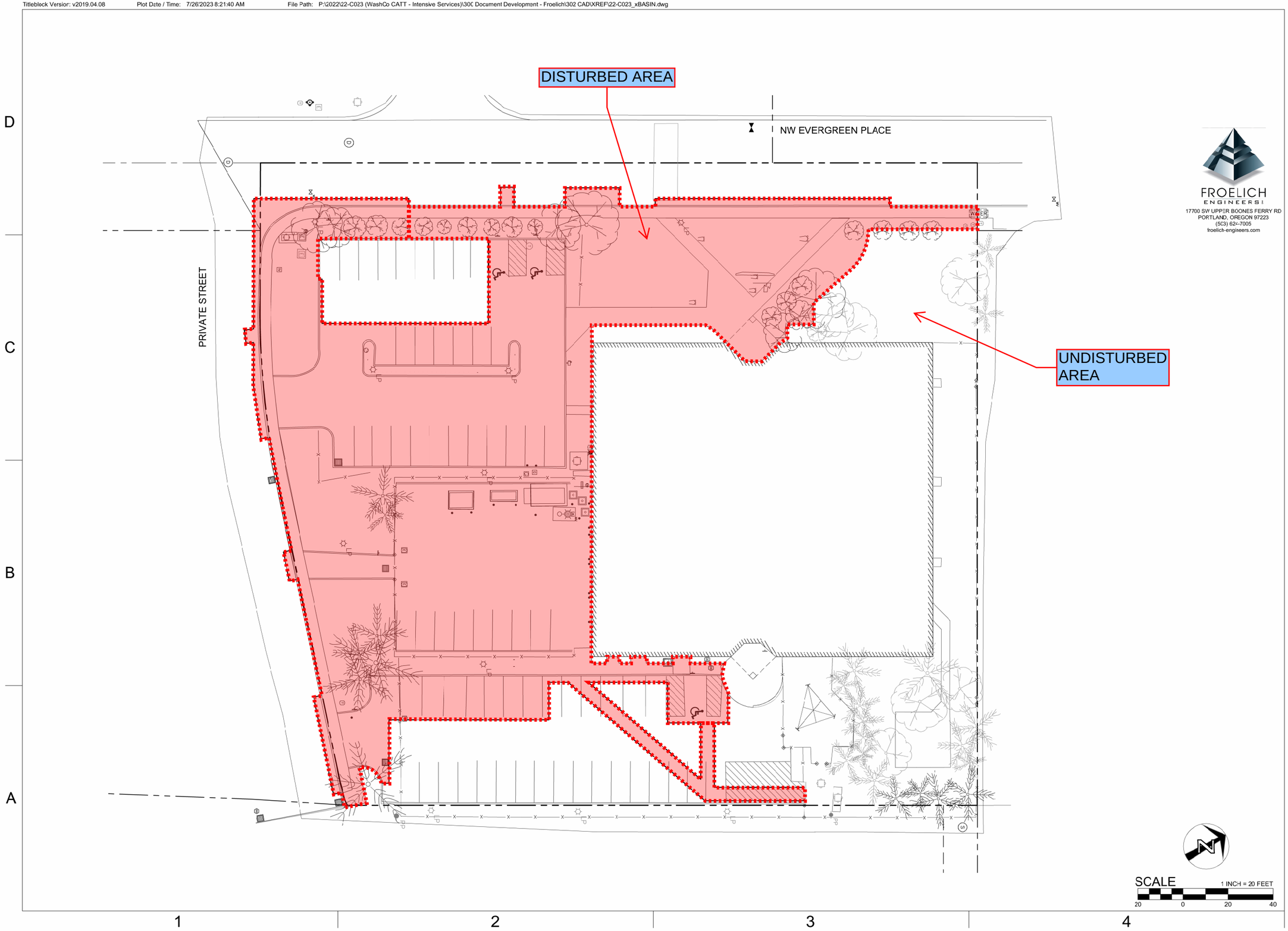
Stormwater Management Requirements
Existing

Basin	Area				Total	
	Pervious		Impervious - Disturbed			
	sf	ac	sf	ac	sf	ac
Existing	13,628	0.31	25,458	0.58	39,086	0.90
Total	13,628	0.31	25,458	0.58	39,086	0.90

Proposed

Basin	Area						Total		Flow Q (25-YR)
	Pervious		Impervious - New		Impervious - Modified				
	sf	ac	sf	ac	sf	ac	sf	ac	cfs
Site	8,895	0.20	8,262	0.24	21,929	0.19	39,086	0.90	0.737
Total	8,895	0.20	8,262	0.24	21,929	0.19	39,086	0.90	

Required Hydromodification = New Impervious + Modified Impervious = 8,262 + 21,929 = 30,191 sf
Required Treatment = New Impervious + 3 * (Modified Impervious - New Pervious) = 8,262 + 3 * (21,929 - 3,529) = 63,462 sf



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Support Services
Facilities and Parks Services Division
Capital Improvement Projects

169 N 1st Ave
Hillsboro, OR. 97124

503-846-8751
MFLcdlfeedExternal@co.washington.or.us
www.co.washington.or.us

REGISTERED PROFESSIONAL ENGINEER OREGON JULY 27, 2010 PEYTON MEYKELBOSCH
PRELIMINARY NOT FOR CONSTRUCTION
RENEWS: 12/31/

ARCHITECT PROJECT NUMBER:
22-010

HOLST

123 NE 3RD AVE.
SUITE 310
PORTLAND, OR
97232

HOLSTARC.COM

COUNTY PROJECT NUMBER:
2040

COUNTY PROJECT MANAGER:
STUART SPAFFORD

BUILDING NUMBER:
0371A

BUILDING NAME:
EVERGREEN (EVGR) - INTENSIVE SERVICES

DEPARTMENT NAME:
HEALTH AND HUMAN SERVICES

PROJECT NAME:
CATT - INTENSIVE SERVICES BUILDING (ISB)

PROJECT ADDRESS:
17511 N/W EVERGREEN PLACE
BEAVERTON, OR 97006

REVISION / ISSUE SCHEDULE

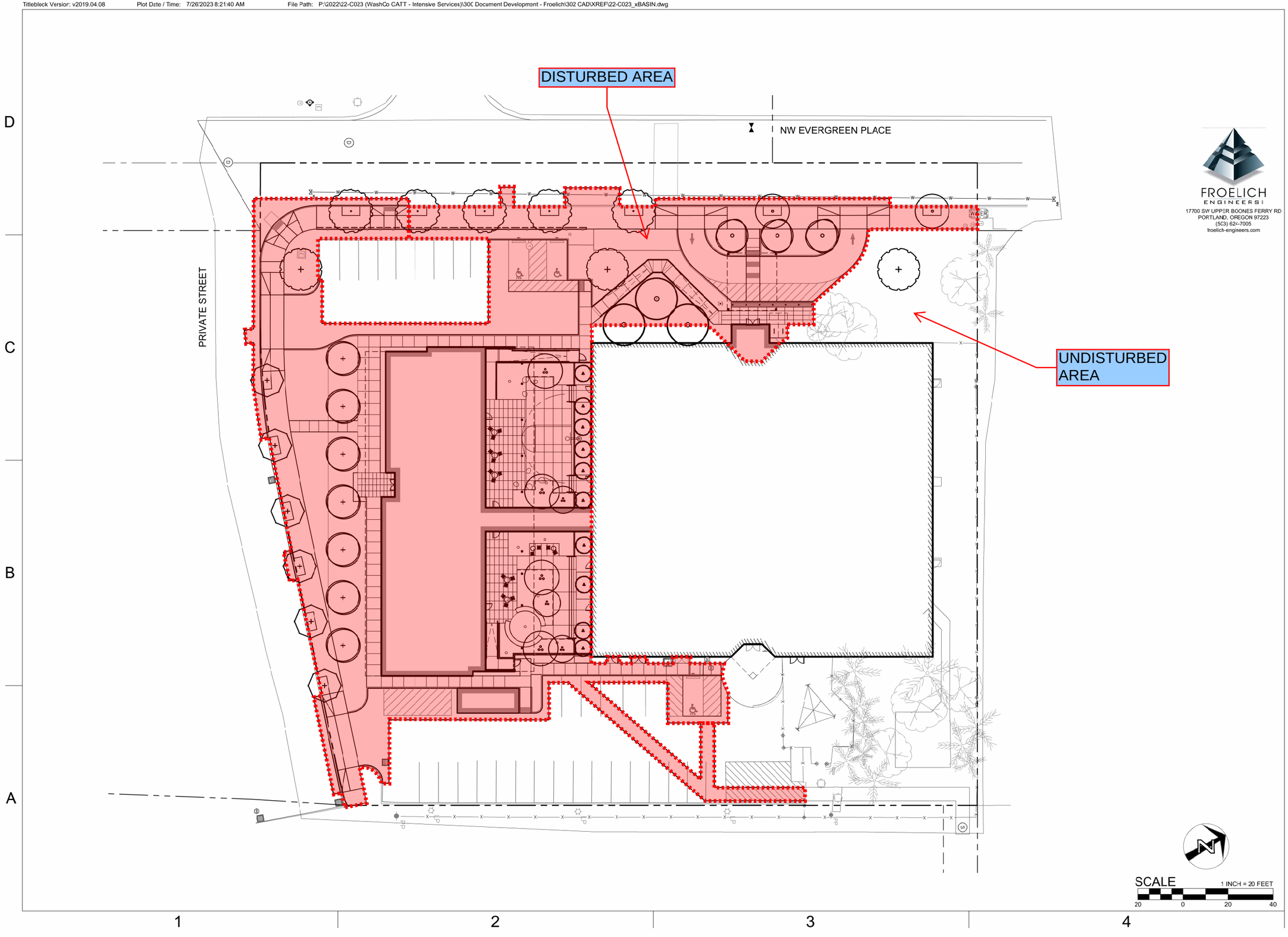
NO.	DESCRIPTON	DATE
A	SCHEMATIC DESIGN	11/18/22
B	50% DESIGN DEVELOPMENT	02/24/23
C	100% DESIGN DEVELOPMENT	04/21/23
D	DESIGN REVIEW	05/23/23
E	DESIGN REVIEW R1	09/01/23

SHEET NUMBER:
EX-1

SHEET NAME:
EXISTING BASIN MAP

DRAWN BY: BLU
SHEET ISSUE DATE: 04/19/23
CHECKED BY: EME

STATUS:
DESIGN REVIEW



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D	DESIGN REVIEW	05/22/23
E	DESIGN REVIEW R1	09/01/23

SHEET NUMBER:
EX-2

SHEET NAME:
PROPOSED BASIN MAP

DRAWN BY: BLU
CHECKED BY: EME
SHEET ISSUE DATE: 04/19/23

STATUS:
DESIGN REVIEW

Appendix C: Assumptions

FROELICH ENGINEERS

Santa Barbara Unit Hydroragh (SBUH) Assumptions:

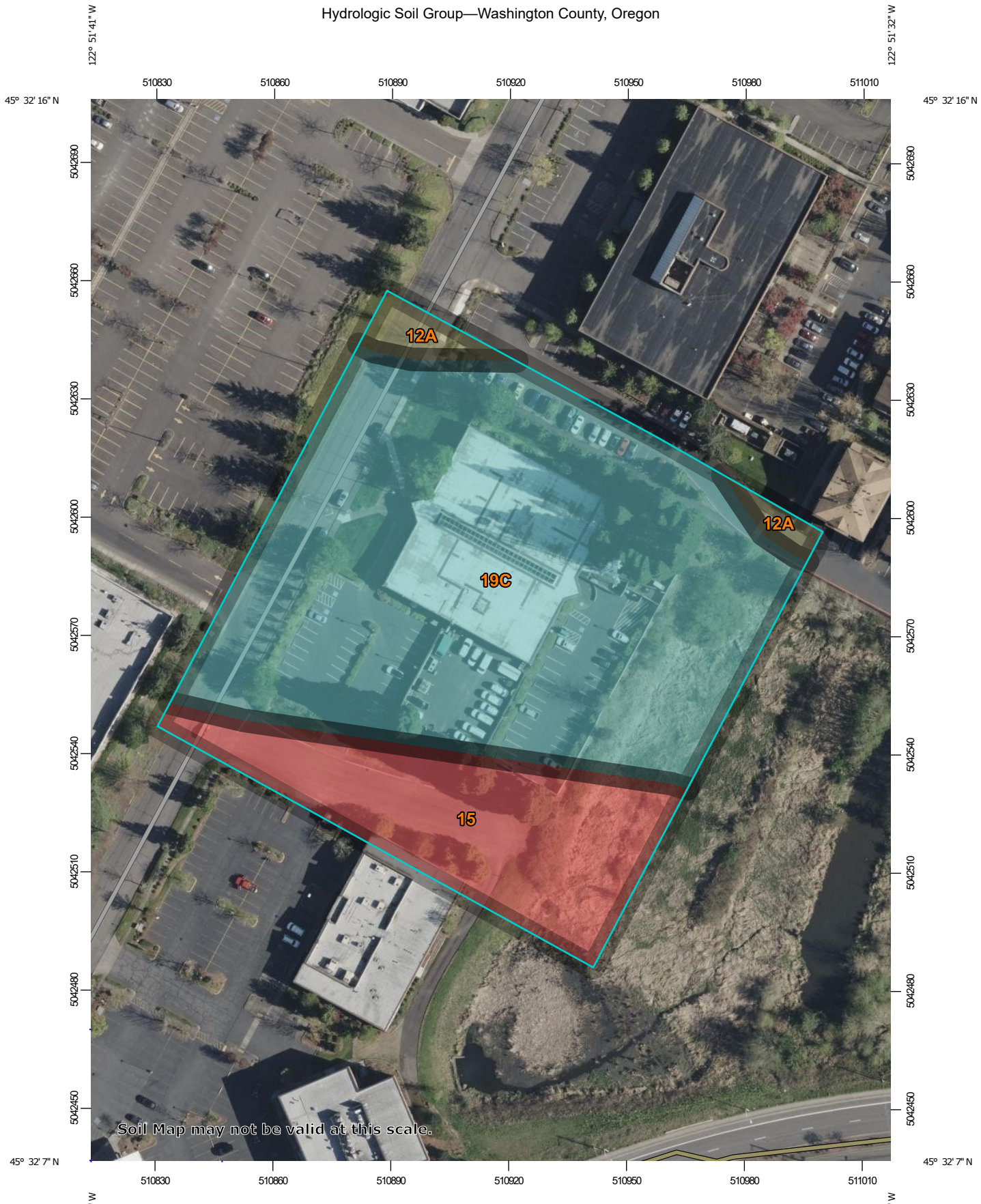
WQ Storm Event=	**0.36 in x Drainage Area (3 hour event)**	
2-year Storm Event=	**2.50**	in/24-hours per CWS Design Standards 12/19
5-year Storm Event=	**3.10**	in/24-hours per CWS Design Standards 12/19
10-year Storm Event=	**3.45**	in/24-hours per CWS Design Standards 12/19
25-year Storm Event =	**3.90**	in/24-hours per CWS Design Standards 12/19
Time of Concentration	**5.0**	minutes
Roughness Coefficient	**0.013**	

Curve Number Assumptions:

Impervious Area =	**98**	Per NRCS
Pervious Area =	**74**	Per NRCS
Existing Pervious Area=	**75**	per CWS Design Standards 12/19

NRCS Soil Group **C**

Appendix D: Soil Resource Report



Map Scale: 1:1,310 if printed on A portrait (8.5" x 11") sheet.

N

Meters
0 15 30 60 90

Feet
0 50 100 200 300

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 10N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

Soil Rating Polygons

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Washington County, Oregon
Survey Area Data: Version 22, Sep 14, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Apr 16, 2021—Apr 18, 2021

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
12A	Cornelius variant silt loam, 0 to 3 percent slopes	C/D	0.1	3.3%
15	Dayton silt loam	D	0.9	22.8%
19C	Helvetia silt loam, 7 to 12 percent slopes	C	2.9	73.9%
Totals for Area of Interest			**3.9**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

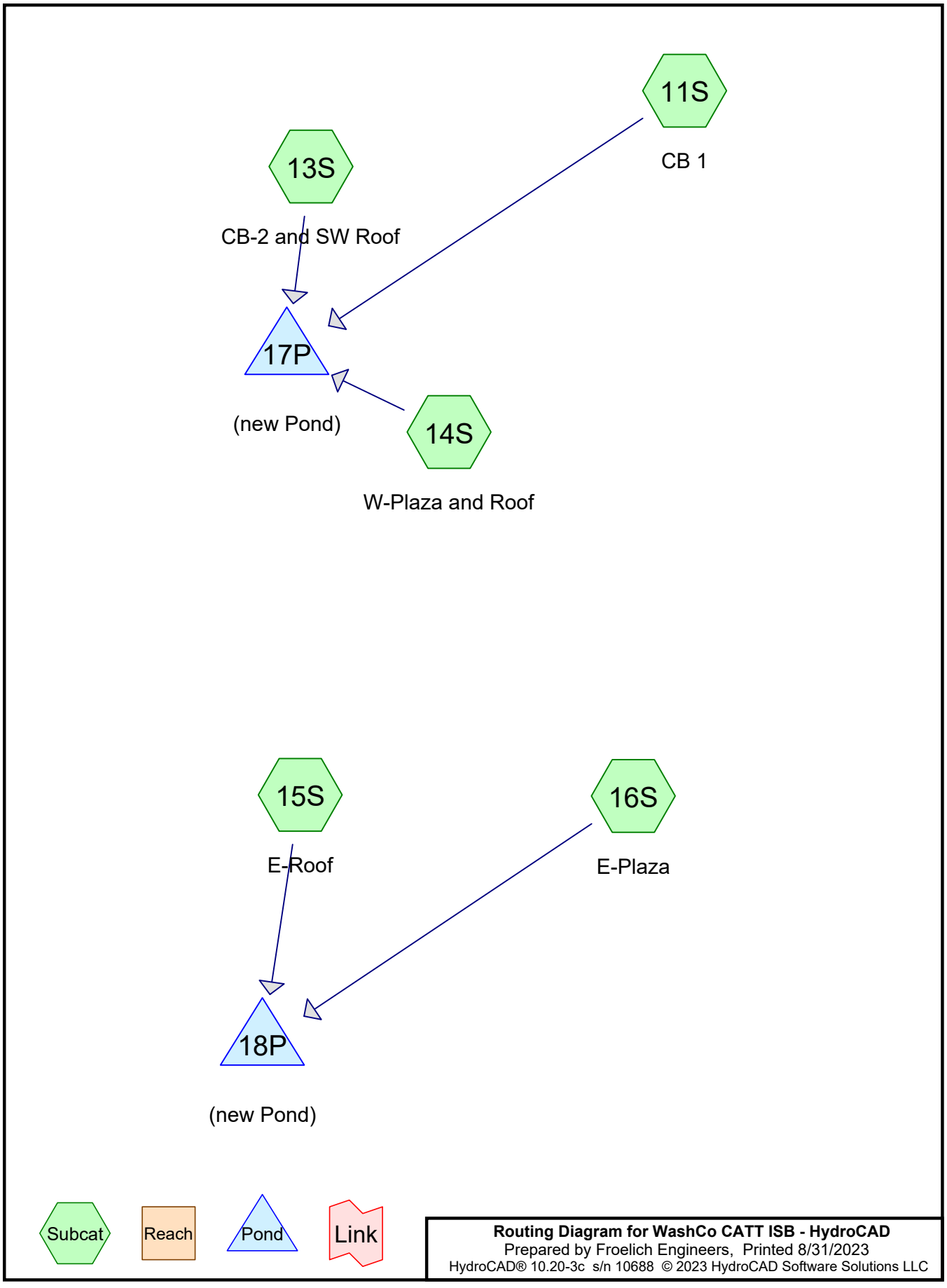
If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Appendix E: Conveyance Calculations

CONVEYANCE CALCULATIONS

DESIGN SECTION		DESIGN CALCULATIONS		Detail						
		RUNOFF	TOTAL	SLOPE	DIA	CAPACITY	VEL.	RUNOFF	VEL.	CAPACITY
PIPE	CONTRIBUTING BASIN/PIPE	Q CFS	Q CFS	%	IN.	Qf CFS	Vf FT/S	RATIO Q/Qf	AT Q/QF	Q VS. Qf
1	CB-1	0.06	0.06	2	6	0.79	4.04	0.07	2.42	OK
2	W-Plaza, W-Roof	0.11	0.11	2	4	0.27	3.08	0.39	2.88	OK
3	Pipe 1,2, SW-Roof, CB-2	0.42	0.58	2	6	0.79	4.04	0.73	4.41	OK
4	E-Roof	0.11	0.11	2	4	0.27	3.08	0.41	2.91	OK
5	E-Plaza	0.02	0.02	2	4	0.27	3.08	0.08	1.85	OK
	Overall W-Site		0.58	2	6	0.79	4.04	0.73	4.41	OK
	Overall E-Site		0.13	2	4	0.27	3.08	0.49	3.07	OK

Catch Basins all use a 6" lead with 2.00% slope which has a design capacity of 0.79 cfs



Routing Diagram for WashCo CATT ISB - HydroCAD
Prepared by Froelich Engineers, Printed 8/31/2023

Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	25-Year	Type IA 24-hr		Default	24.00	1	3.90	2

Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment11S: CB 1 Runoff Area=3,000 sf 76.23% Impervious Runoff Depth=3.02"
Tc=5.0 min CN=92 Runoff=0.055 cfs 755 cf

Subcatchment13S: CB-2 and SW Roof Runoff Area=22,148 sf 79.84% Impervious Runoff Depth=3.12"
Tc=5.0 min CN=93 Runoff=0.418 cfs 5,763 cf

Subcatchment14S: W-Plaza and Roof Runoff Area=5,222 sf 85.68% Impervious Runoff Depth=3.33"
Tc=5.0 min CN=95 Runoff=0.105 cfs 1,450 cf

Subcatchment15S: E-Roof Runoff Area=5,143 sf 100.00% Impervious Runoff Depth=3.67"
Tc=5.0 min CN=98 Runoff=0.110 cfs 1,571 cf

Subcatchment16S: E-Plaza Runoff Area=1,576 sf 42.51% Impervious Runoff Depth=2.28"
Tc=5.0 min CN=84 Runoff=0.021 cfs 300 cf

Pond 17P: (new Pond) Inflow=0.577 cfs 7,969 cf
Primary=0.577 cfs 7,969 cf

Pond 18P: (new Pond) Inflow=0.130 cfs 1,871 cf
Primary=0.130 cfs 1,871 cf

Total Runoff Area = 37,089 sf Runoff Volume = 9,840 cf Average Runoff Depth = 3.18"
18.42% Pervious = 6,833 sf 81.58% Impervious = 30,256 sf

Summary for Subcatchment 11S: CB 1

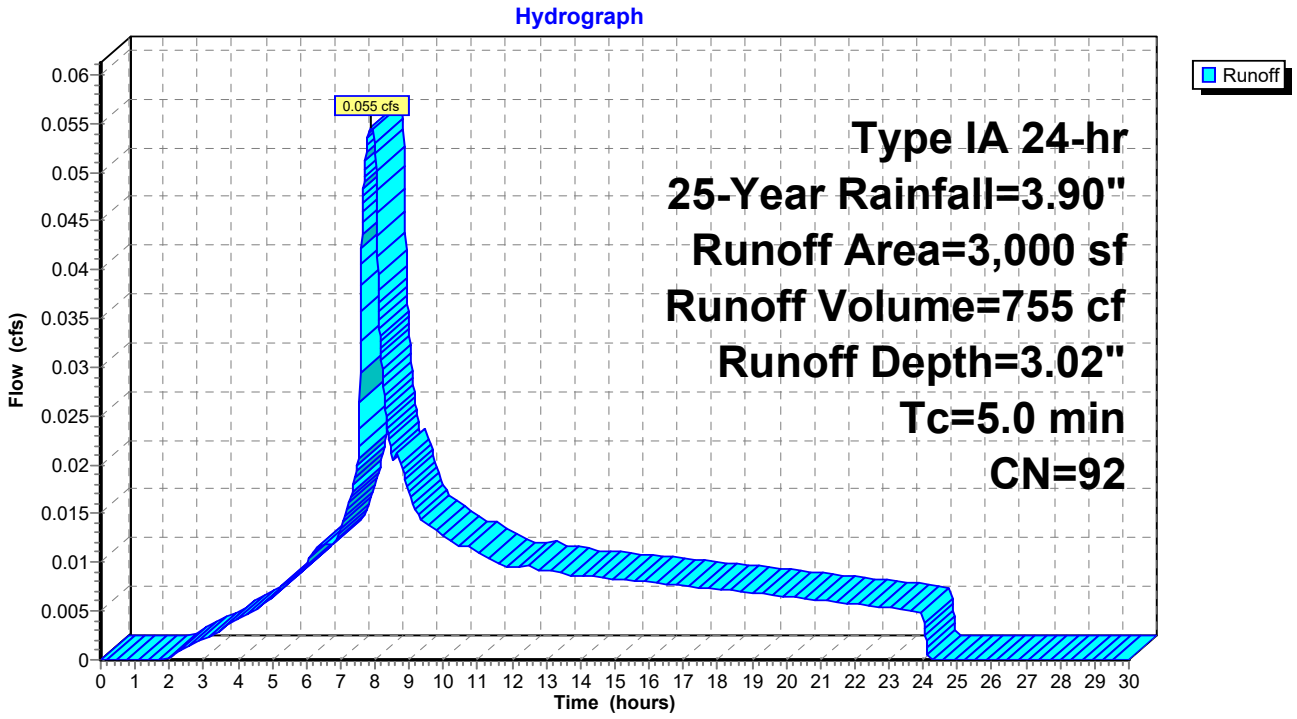
Runoff = 0.055 cfs @ 7.89 hrs, Volume= 755 cf, Depth= 3.02"
Routed to Pond 17P : (new Pond)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type IA 24-hr 25-Year Rainfall=3.90"

	Area (sf)	CN	Description
*	2,287	98	Impervious West
*	172	74	
*	372	74	
*	169	74	
	3,000	92	Weighted Average
	713		23.77% Pervious Area
	2,287		76.23% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 11S: CB 1



Summary for Subcatchment 13S: CB-2 and SW Roof

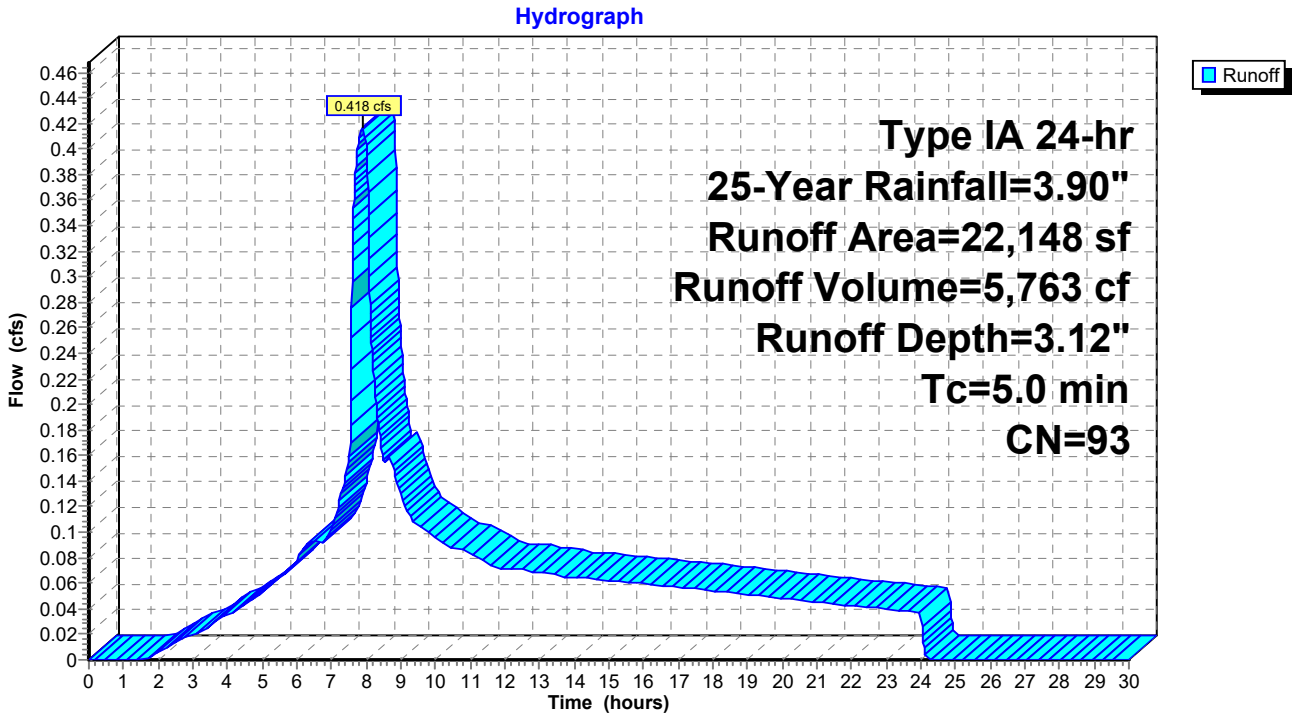
Runoff = 0.418 cfs @ 7.88 hrs, Volume= 5,763 cf, Depth= 3.12"
Routed to Pond 17P : (new Pond)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type IA 24-hr 25-Year Rainfall=3.90"

	Area (sf)	CN	Description
*	17,682	98	Impervious West
*	1,183	74	
*	312	74	
*	2,971	74	
	22,148	93	Weighted Average
	4,466		20.16% Pervious Area
	17,682		79.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 13S: CB-2 and SW Roof



Summary for Subcatchment 14S: W-Plaza and Roof

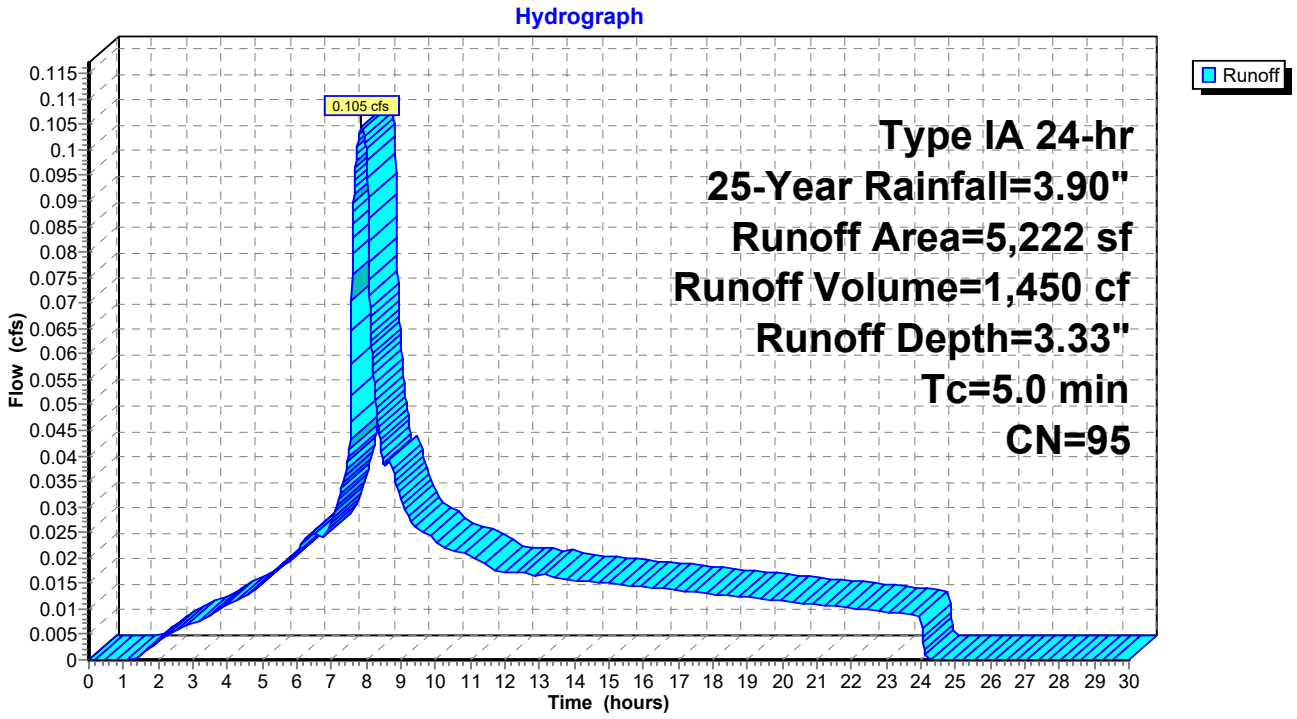
Runoff = 0.105 cfs @ 7.87 hrs, Volume= 1,450 cf, Depth= 3.33"
Routed to Pond 17P : (new Pond)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type IA 24-hr 25-Year Rainfall=3.90"

	Area (sf)	CN	Description
*	4,474	98	Impervious West
*	212	74	
*	246	74	
*	290	74	
	5,222	95	Weighted Average
	748		14.32% Pervious Area
	4,474		85.68% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 14S: W-Plaza and Roof



Summary for Subcatchment 15S: E-Roof

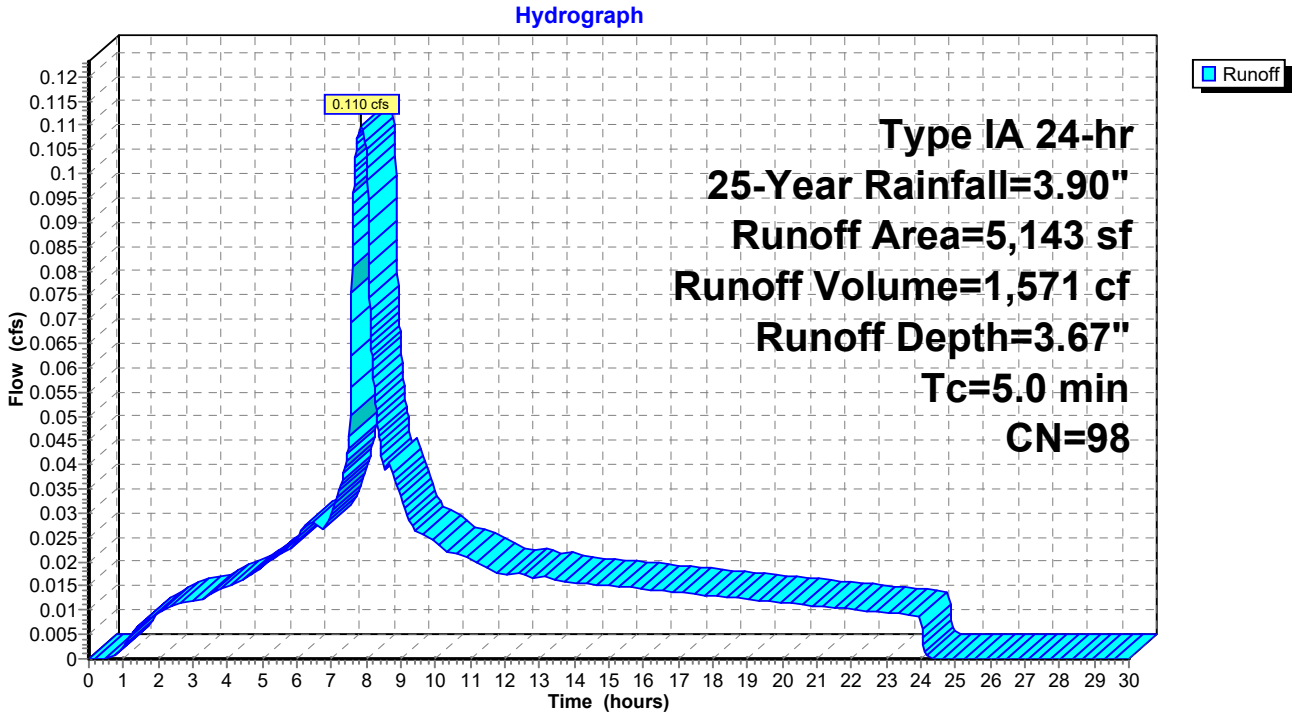
Runoff = 0.110 cfs @ 7.86 hrs, Volume= 1,571 cf, Depth= 3.67"
Routed to Pond 18P : (new Pond)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type IA 24-hr 25-Year Rainfall=3.90"

	Area (sf)	CN	Description
*	5,143	98	Impervious West
	5,143		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 15S: E-Roof



Summary for Subcatchment 16S: E-Plaza

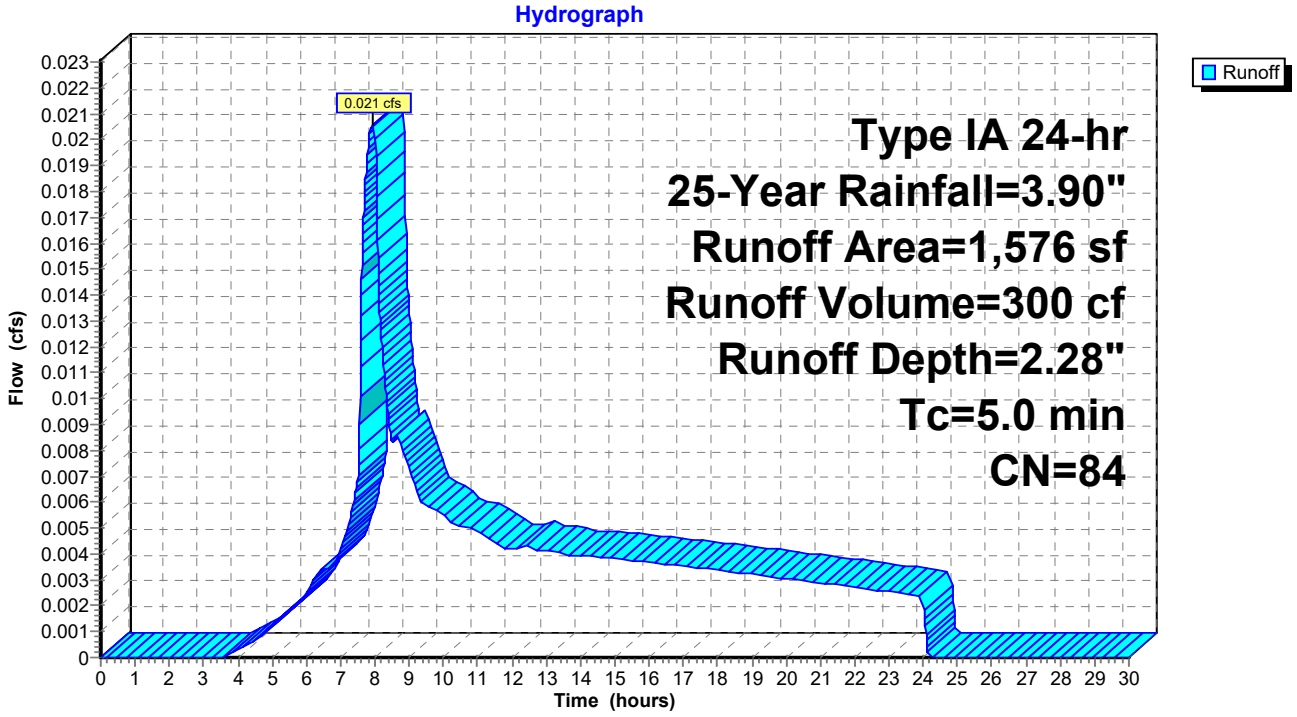
Runoff = 0.021 cfs @ 7.94 hrs, Volume= 300 cf, Depth= 2.28"
Routed to Pond 18P : (new Pond)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type IA 24-hr 25-Year Rainfall=3.90"

	Area (sf)	CN	Description
*	670	98	Impervious West
*	400	74	
*	209	74	
*	81	74	
*	216	74	
	1,576	84	Weighted Average
	906		57.49% Pervious Area
	670		42.51% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 16S: E-Plaza

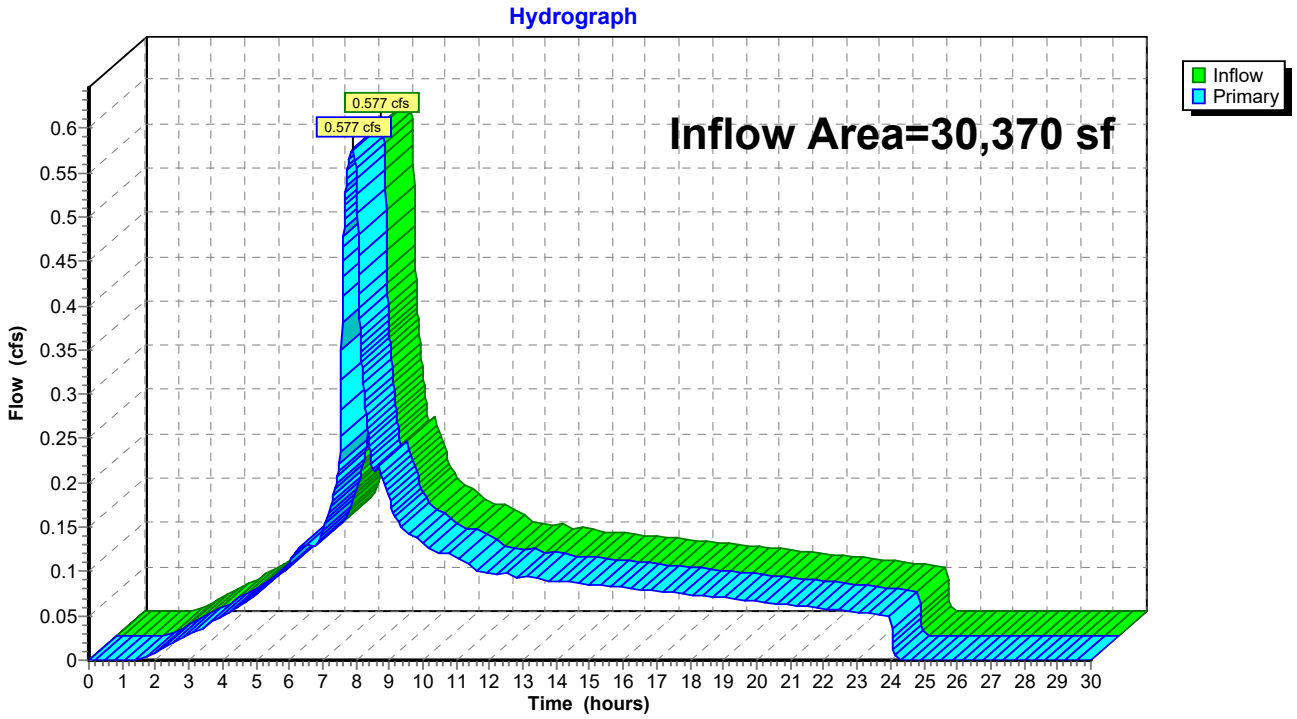


Summary for Pond 17P: (new Pond)

Inflow Area = 30,370 sf, 80.48% Impervious, Inflow Depth = 3.15" for 25-Year event
Inflow = 0.577 cfs @ 7.88 hrs, Volume= 7,969 cf
Primary = 0.577 cfs @ 7.88 hrs, Volume= 7,969 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Pond 17P: (new Pond)

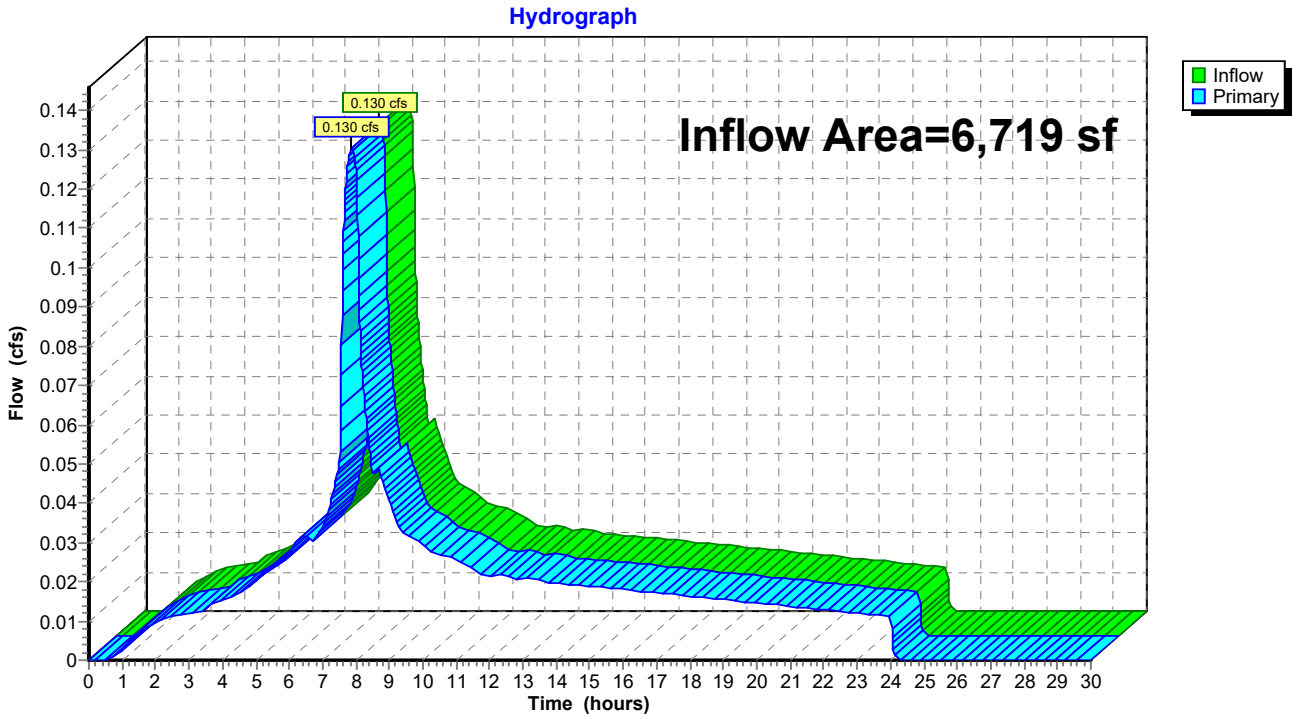


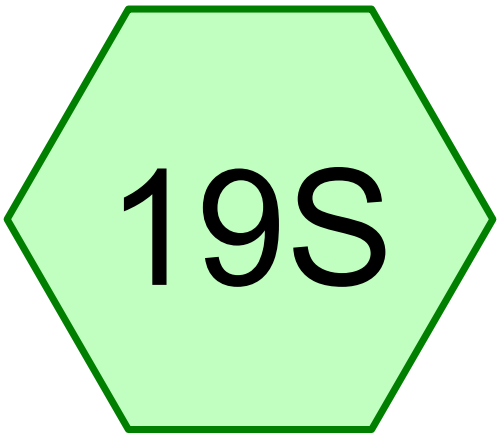
Summary for Pond 18P: (new Pond)

Inflow Area = 6,719 sf, 86.52% Impervious, Inflow Depth = 3.34" for 25-Year event
Inflow = 0.130 cfs @ 7.87 hrs, Volume= 1,871 cf
Primary = 0.130 cfs @ 7.87 hrs, Volume= 1,871 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Pond 18P: (new Pond)





Site Flow

Subcat Reach Pond Link

Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	25-Year	Type IA 24-hr		Default	24.00	1	3.90	2

Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment19S: Site Flow Runoff Area=39,086 sf 77.24% Impervious Runoff Depth=3.12"
Tc=5.0 min CN=93 Runoff=0.737 cfs 10,171 cf

Total Runoff Area = 39,086 sf Runoff Volume = 10,171 cf Average Runoff Depth = 3.12"
22.76% Pervious = 8,895 sf 77.24% Impervious = 30,191 sf

Summary for Subcatchment 19S: Site Flow

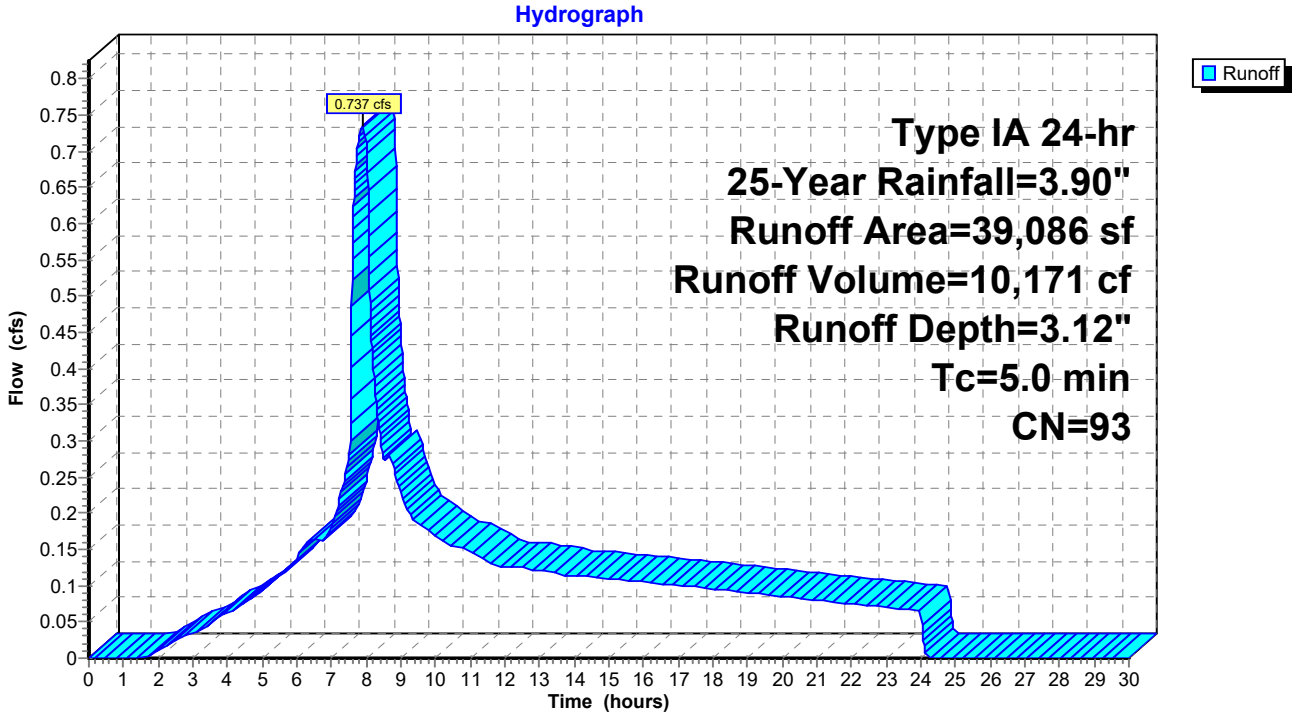
Runoff = 0.737 cfs @ 7.88 hrs, Volume= 10,171 cf, Depth= 3.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs
Type IA 24-hr 25-Year Rainfall=3.90"

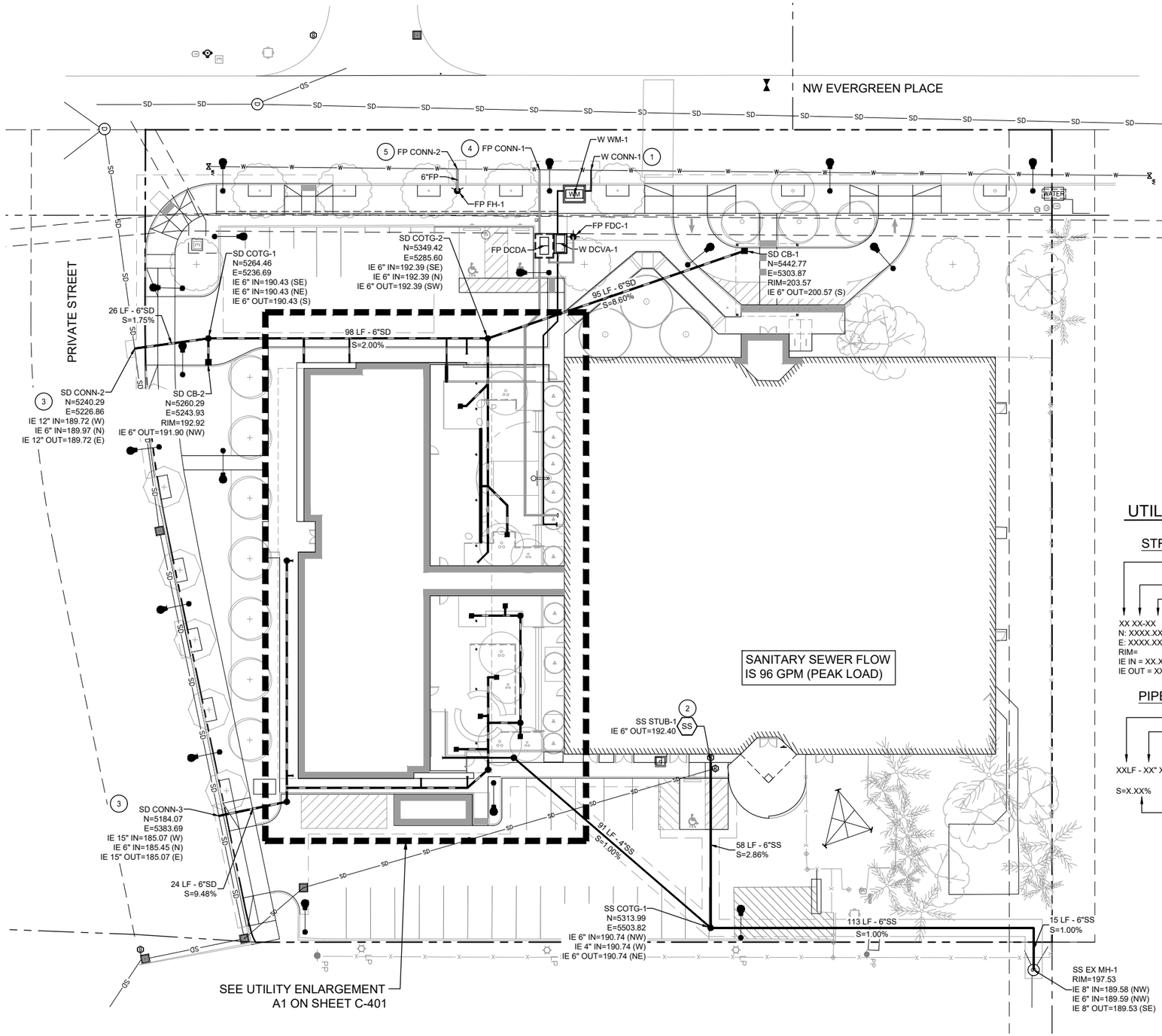
Area (sf)	CN	Description
8,895	74	>75% Grass cover, Good, HSG C
8,262	98	Paved parking, HSG C
21,929	98	Paved parking, HSG C
39,086	93	Weighted Average
8,895		22.76% Pervious Area
30,191		77.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Subcatchment 19S: Site Flow



Appendix F: Utility Plan / Details



SHEET NOTES

1. PIPE BEDDING AND BACKFILL FOR ALL UTILITIES SHALL BE DONE PER DETAIL A1/C-500
2. STRUCTURES LOCATIONS ARE BASED ON CENTER OF STRUCTURE.
3. INSTALL THRUST BLOCK ON FIRE AND WATER LINES.
4. CONTRACTOR TO VERIFY TIE-IN ELEVATION AND COMMUNICATE ANY DISCREPANCIES TO THE ENGINEER OF RECORD.
5. CONNECTIONS TO ANY STRUCTURE MUST USE AN APPROVED WATERTIGHT SEAL. THIS INCLUDES MANHOLES, CATCH BASINS, VAULTS, AREA DRAINS, DITCH INLETS, OR OTHERS. REFERENCE DETAIL A3/C-501. ANY VARIATION MUST BE PRE-APPROVED BY THE CITY OF BEAVERTON ENGINEER. GLUE AND SAND IS NOT ACCEPTABLE.

(X) KEY NOTES

1. COORDINATE WATER SERVICE POINT OF CONNECTION FOR PROPOSED 3" WATER METER. COORDINATE WITH CITY OF BEAVERTON.
2. REPLACE EXISTING 6" SANITARY SEWER LATERAL WITH PROPOSED 6" SEWER LATERAL USING TRENCHLESS TECHNOLOGY. CONNECT TO EXISTING BUILDING STUB AND EXISTING SANITARY SEWER MANHOLE.
3. FIELD VERIFY LOCATION, SIZE, AND IE OF EXISTING STORM MAIN. CONNECT PROPOSED LATERAL TO MAIN WITH PVC GASKETED SADDLE PER CITY STD DETAIL C2/C-501.
4. COORDINATE FIRE PROTECTION SERVICE POINT OF CONNECTION. COORDINATE WITH CITY OF BEAVERTON.
5. COORDINATE FIRE HYDRANT POINT OF CONNECTION. COORDINATE WITH CITY OF BEAVERTON.

UTILITY LABEL LEGEND

STRUCTURE LABEL

UTILITY TYPE (SD=STORM DRAINAGE)
STRUCTURE TYPE CALLOUT
ID NUMBER (WHERE APPLICABLE)

XX.XX-XX
N: XXXX.XX
E: XXXX.XX
RIM=
IE IN = XX.X
IE OUT = XX.X

LOCATION (WHERE APPLICABLE)
STRUCTURE INFO (WHERE APPLICABLE)

PIPE LABEL

UTILITY LENGTH
UTILITY SIZE
UTILITY TYPE

XXLF - XX" XX

S=X.XX%

SLOPE (WHERE APPLICABLE)

STRUCTURE TYPE

CALLOUT	DESCRIPTION	DETAIL REF.
AD	AREA DRAIN	D2/C504
BEND	BEND, USE FITTING IF APPL.	
BWV	BACKWATER VALVE	A1/C504
CB	TRAPPED CATCH BASIN	B2/C504
CONN	CONNECTION	
COTG	CLEANOUT TO GRADE	C1/C504
DCDA	DOUBLE CHECK DETECTOR	A3/C502 C2/C502
DCVA	DOUBLE CHECK VALVE	A1/C501 C3/C502
FD	FOUNDATION DRAINAGE POINT	
FDC	FIRE DEPARTMENT CONNECTION	A2/C504
FH	FIRE HYDRANT	A2/C501
GV	GATE VALVE	
MH	MANHOLE	
TEE	TEE CONNECTION	
TD	TRENCH DRAIN	B1/C504
WM	WATER METER	C1/C501
WYE	WYE CONNECTION	

SHEET LEGEND

- RD — CONNECT TO STORM DRAIN/ROOF DRAIN. SEE PLUMBING PLANS FOR CONTINUATION. SIZE AND IE AS NOTED.
- FP — CONNECT TO FIRE PROTECTION SYSTEM. SIZE AS NOTED. SEE PLUMBING PLANS FOR CONTINUATION.
- S — CONNECT TO WASTE LINE. SEE PLUMBING PLANS FOR CONTINUATION. SIZE AS NOTED.
- DCV — DOUBLE CHECK VAULT — A1/C501 C3/C502
- DCD — DOUBLE CHECK DETECTOR VAULT — A3/C502 C2/C502
- W — CONNECT TO COLD WATER SYSTEM. SEE PLUMBING PLANS FOR CONTINUATION. SIZE AS NOTED.

SCALE 1 INCH = 20 FEET
20 0 20 40

A1 UTILITY PLAN
SCALE: 1" = 20'

FROELICH ENGINEERS
17700 SW UPPER BOONES FERRY RD
PORTLAND, OREGON 97223
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froelich-engineers.com

Washington County
Support Services
Facilities and Parks Services Division
Capital Improvement Projects

169 N 1st Ave
Hillsboro, OR. 97124

503-846-8751
MFacilBeeExternal@co.washington.or.us
www.co.washington.or.us

REGISTERED PROFESSIONAL ENGINEER OREGON JULY 10, 2018 PEYTON M. EYKELBOSCH
PRELIMINARY NOT FOR CONSTRUCTION
RENEWS: 12/31/

ARCHITECT PROJECT NUMBER:
22-010

HOLST

123 NE 3RD AVE.
SUITE 310
PORTLAND, OR
97232

HOLSTARC.COM

COUNTY PROJECT NUMBER:
2040

COUNTY PROJECT MANAGER:
STUART SPAFFORD

BUILDING NUMBER:
0371A

BUILDING NAME:
EVERGREEN (EVGR) - INTENSIVE SERVICES

DEPARTMENT NAME:
HEALTH AND HUMAN SERVICES

PROJECT NAME:
CATT - INTENSIVE SERVICES BUILDING (ISB)

PROJECT ADDRESS:
17911 NW EVERGREEN PLACE
BEAVERTON, OR 97006

REVISION / ISSUE SCHEDULE

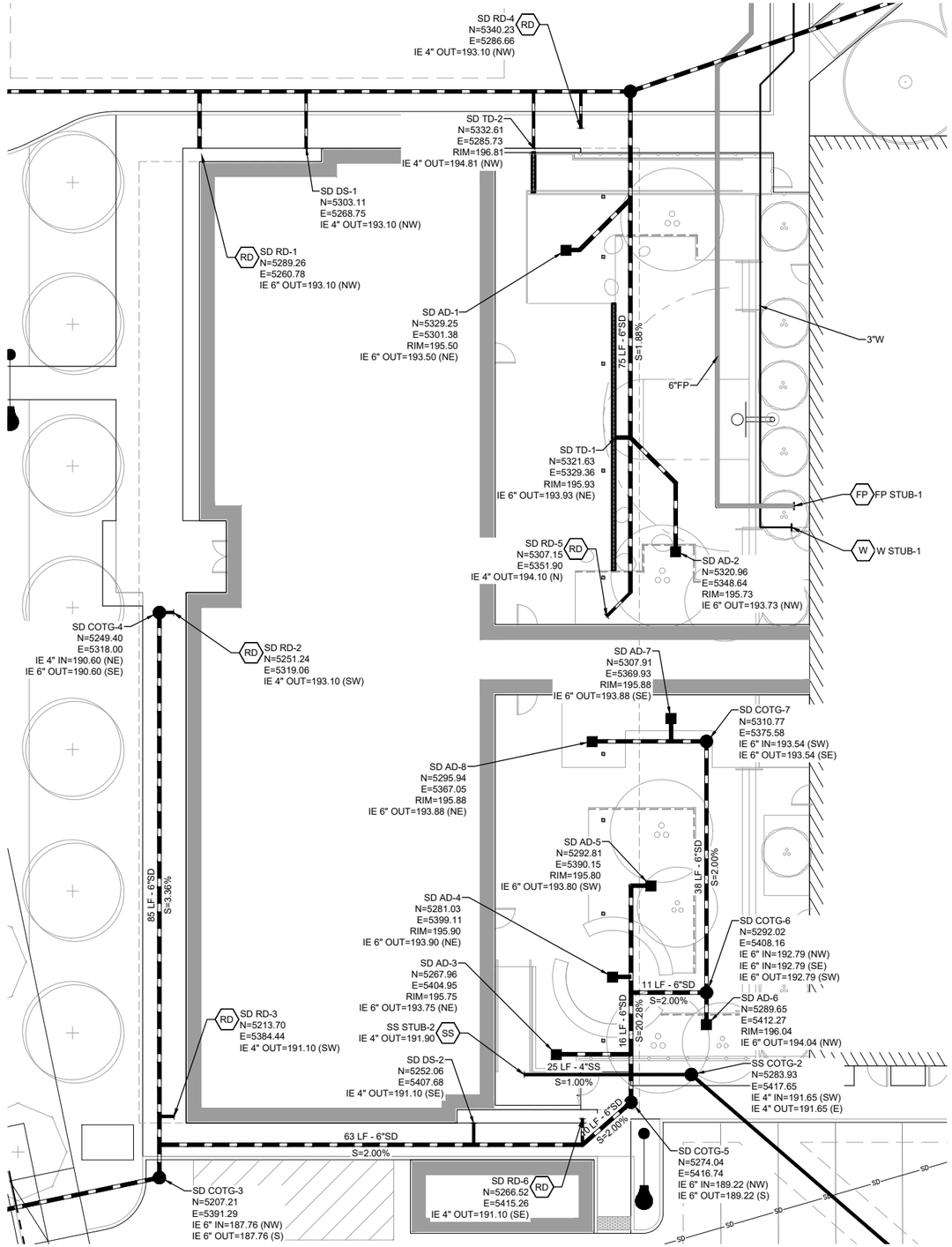
NO.	DESCRIPTION	DATE
A	SCHEMATIC DESIGN	11/18/22
B	50% DESIGN DEVELOPMENT	02/24/23
C	100% DESIGN DEVELOPMENT	04/21/23
D	DESIGN REVIEW	05/26/23
E	DESIGN REVIEW R1	09/01/23

SHEET NUMBER:
C-400

SHEET NAME:
UTILITY PLAN

DRAWN BY: **BLU**
SHEET ISSUE DATE: **04/19/23**
CHECKED BY: **EME**

STATUS:
DESIGN REVIEW



A1 UTILITY ENLARGEMENT
SCALE: 1" = 5'

SCALE 1 INCH = 10 FEET
10 0 10 20

FROELICH ENGINEERS
17700 SW UPPER BOONES FERRY RD
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Washington County
Support Services
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Capital Improvement Projects

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www.co.washington.or.us

REGISTERED PROFESSIONAL ENGINEER OREGON JULY 31, 2010 PREVAN VAN EYKELBOSCH
PRELIMINARY NOT FOR CONSTRUCTION
RENEWS: 12/31/

ARCHITECT PROJECT NUMBER:
22-010

HOLST
123 NE 3RD AVE.
SUITE 310
PORTLAND, OR
97232

HOLSTARC.COM

COUNTY PROJECT NUMBER:
2040

COUNTY PROJECT MANAGER:
STUART SPAFFORD

BUILDING NUMBER:
0371A

BUILDING NAME:
EVERGREEN (EVGR) - INTENSIVE SERVICES

DEPARTMENT NAME:
HEALTH AND HUMAN SERVICES

PROJECT NAME:
CATT - INTENSIVE SERVICES BUILDING (ISB)

PROJECT ADDRESS:
17911 NW EVERGREEN PLACE
BEAVERTON, OR 97006

REVISION / ISSUE SCHEDULE

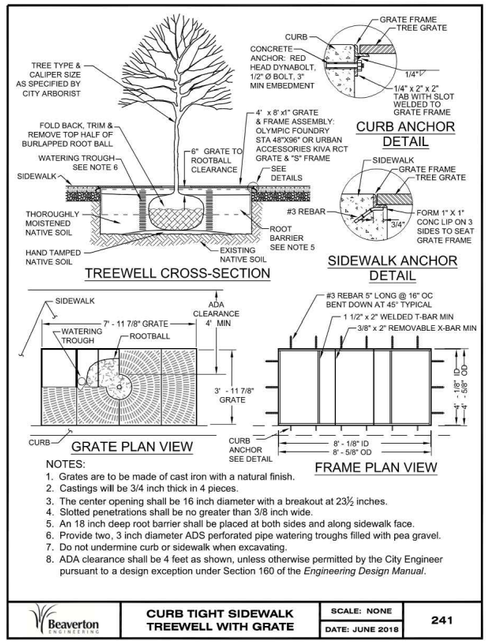
NO.	DESCRIPTION	DATE
A	SCHEMATIC DESIGN	11/18/22
B	50% DESIGN DEVELOPMENT	02/24/23
C	100% DESIGN DEVELOPMENT	04/21/23
D	DESIGN REVIEW	05/26/23
E	DESIGN REVIEW R1	09/01/23

SHEET NUMBER:
C-401

SHEET NAME:
UTILITY ENLARGEMENT

DRAWN BY: **BLU**
SHEET ISSUE DATE: **04/19/23**
CHECKED BY: **EME**

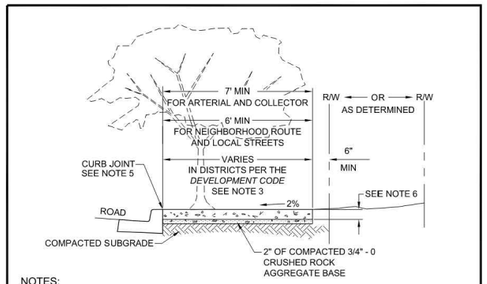
STATUS:
DESIGN REVIEW



NOTES:
1. Grates are to be made of cast iron with a natural finish.
2. Castings will be 3/4 inch thick in 4 pieces.
3. The center opening shall be 16 inch diameter with a breakout at 23½ inches.
4. Slotted penetrations shall be no greater than 3/8 inch wide.
5. An 18 inch deep root barrier shall be placed at both sides and along sidewalk face.
6. Provide two, 3 inch diameter ADS perforated pipe watering troughs filled with pea gravel.
7. Do not undermine curb or sidewalk when excavating.
8. ADA clearance shall be 4 feet as shown, unless otherwise permitted by the City Engineer pursuant to a design exception under Section 160 of the *Engineering Design Manual*.

Beaverton Engineering	CURB TIGHT SIDEWALK TREEWELL WITH GRATE	SCALE: NONE DATE: JUNE 2018	241

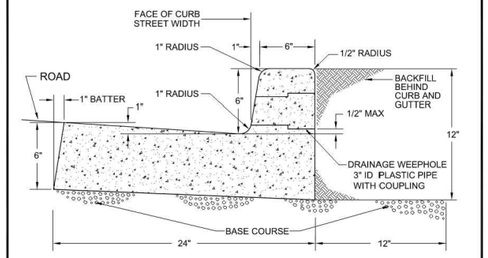
C1 CURB TIGHT S/W TREEWELL W/ GRATE
SCALE: NTS



NOTES:
1. Curb tight sidewalks **REQUIRE PREAPPROVAL** by the City Engineer. They are used for sidewalk repairs, replacements and installations in existing developments. Match width of existing sidewalks, and widths and lengths of existing sidewalk panels.
2. Concrete shall have a minimum compressive strength of 4,000 psi at 28 days, For slump see specifications.
3. Sidewalk panels shall be square with their length equal to the sidewalk's width, except that sidewalks in the Regional Center, Town Center, Station Area and Station Community districts may be wider than 6 feet, in which cases their panels may be 4 to 6 feet square, but all of equal size.
4. Expansion joints to be placed at sides of driveway approaches, utility vaults, sidewalk ramps and/or at points of tangency in curb as shown on the standard drawings for sidewalk ramps and at spacing not to exceed 45 feet.
5. For sidewalks adjacent to the curb and poured at the same time as the curb, the joint between them shall be troweled with a minimum 1/2 inch radius.
6. Sidewalk shall have a minimum thickness of 4 inches, except that sidewalk that is intended as a portion of a driveway shall have a minimum thickness of 6 inches. See Drawings 210 & 211.
7. Where vehicular access across sidewalk is required by City, a 40 foot long section of sidewalk shall be provided in the access area, shall be 6-inches thick and shall be reinforced with 6"x6"x10 ga steel mesh. Location of 40 foot long section to be as directed by City Engineer.
8. Finish with broom and edge all joints.
9. Street trees, treewells and grates are required except where specifically modified or waived in writing by the City Engineer.
10. For sidewalk widths around grated treewells, and tree grate requirements, see *Beaverton Standard Dwg* 241.

Beaverton Engineering	CURB TIGHT SIDEWALK	SCALE: NONE DATE: JUNE 2018	216

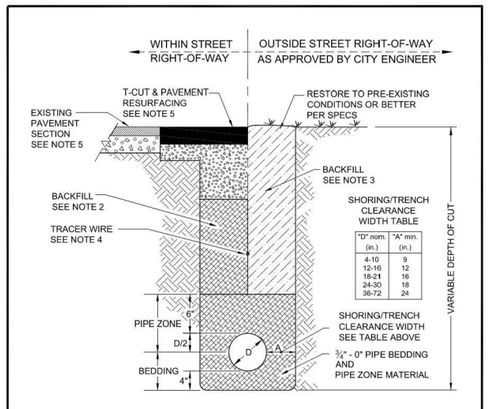
C2 CURB TIGHT SIDEWALK
SCALE: NTS



NOTES:
1. For use along medians, gutters may be reduced when preapproved by City Engineer.
2. Concrete to have compressive strength of 4,000 psi at 28 days.
3. Expansion joints to be provided at each:
   a. Point of tangency.
   b. Cold joint.
   c. Side of inlet structures.
   d. Side of driveways.
4. Expansion joint material to be pre-molded, asphalt impregnated, non-extruding, with a thickness of 1/2 inch.
5. Contraction Joints shall have:
   a. Spacing of not more than 15 feet.
   b. Depth of joint of at least 1½ inches.
6. Base rock 1½"-0 or 3/4"-0, 95% (*AASHTO* T-180) compaction. Base rock shall be to subgrade of street structure or 7.5 inches, whichever is greater, and shall extend 12 inches behind the curb.
7. Drainage weephole shall be:
   a. 3-inch diameter I.D. plastic pipe with coupling and beveled outlet end to match face of curb.
   b. Centered with contraction joints.
   c. Core-drilled through existing curbs for drainage access.
8. Proof roll subgrade and base rock section prior to placement of curb.

Beaverton Engineering	STANDARD MONOLITHIC CURB AND GUTTER	SCALE: NONE DATE: JUNE 2018	205

C3 STD MONOLITHIC CURB AND GUTTER
SCALE: NTS

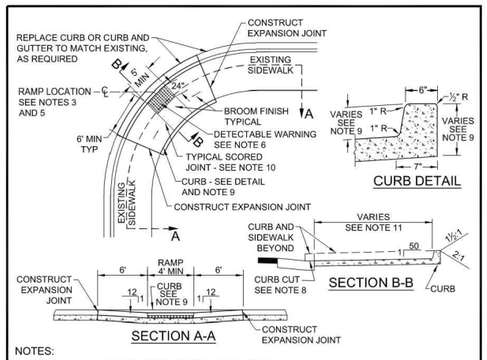


NOTES:
1. These trench backfill requirements apply to all public utility pipes. For additional requirements, see *CWS Design and Construction Standards* and City's *Engineering Design Manual* section 211.
2. For trenches 12" wide or less in streets with classifications higher than Residential, use controlled density fill (CDF) as per sec 210.19. For all other trenches, use 3/4" - 0" aggregate compacted to 95% of max density per *AASHTO* t-99 and *ODOT/APWA spec* 00405.
3. Use class 'A' backfill compacted to 90% of max density per *AASHTO* t-99 and *ODOT/APWA spec* 00405.
4. Tracer wire shall be placed above non-metalic pipe and 2 feet below finished grade when required by Oregon State Plumbing code. Reference City's EDM section 211.1.4.o. Storm and sanitary sewer services shall be marked with magnetic tape per CWS D&C section 5.09.4.c
5. Restore to original conditions or better. See specifications and drawing 302.
6. For conduit trench requirements see drawing 301.
7. The City reserves the right to require compaction testing of the bedding material when it exceeds the 4-inch base section beneath the pipe.

Beaverton Engineering	PIPE TRENCH BACKFILL	SCALE: NONE DATE: JUNE 2018	300

FILENAME: 300.dwg

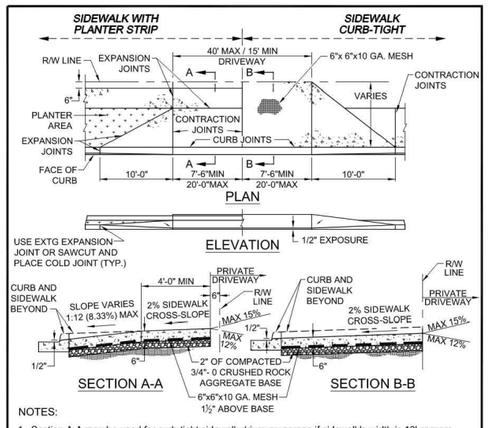
A1 PIPE TRENCH BACKFILL
SCALE: NTS



NOTES:
1. Retrofit ramp **REQUIRES PREAPPROVAL** by the City Engineer.
2. Sidewalk ramp shall meet *ADA Standards*.
3. This drawing is for retrofitted ramps only, where site constraints prohibit installing two ramps. The Engineer shall prepare a site-specific drawing for each ramp, accepting full responsibility for correcting all unacceptable ramp construction resulting from applying this drawing *"as is"*.
4. Sawcut and remove the existing sidewalk, curb and gutter to the nearest cold joint.
5. Each ramp shall be located relative to crosswalk or stop line.
6. Detectable warning shall be 24 inches long in the direction of travel and full width of the ramp, made of concrete imbedded yellow tiles, that have truncated domes aligned on a square grid with its gridlines parallel and perpendicular to the centerline of the ramp, from the approved list in the *Engineering Design Manual, Sec* 210.10.
7. Concrete shall have a compressive strength of 4,000 psi at 28 days.
8. Bevel the curb cut from gutter to back of curb at 8.33% (1:12).
9. Construct curb with varying exposure tapered longitudinally so that the top of the curb matches the normal projected back of sidewalk as shown in section B-B.
10. Score at grade changes, surface texture changes and at other points shown. Edges shall be shined.
11. For sidewalk widths, see *drawing 216*.
12. Curb inlet or catch basin shall not be allowed in front of sidewalk ramp.

Beaverton Engineering	RETROFIT CURB TIGHT SINGLE SIDEWALK RAMP	SCALE: NONE DATE: JUNE 2018	233

A2 RETROFIT CURB TIGHT SINGLE S/W RAMP
SCALE: NTS



NOTES:
1. Section A-A may be used for curb-tight sidewalk driveway aprons if sidewalk's width is 10' or more.
2. Concrete shall have a minimum breaking strength of 4,000 psi after 28 days.
3. Curb joint shall be a troweled joint with a minimum 1/2 inch radius along back of curb.
4. Expansion joints shall be 1/2 inch pre-molded asphalt impregnated material, cedar or approved equal extending from top of base to finished grade.
5. For driveways 24 feet wide or greater, concrete to be increased to a 7 inch depth.
6. Finish with broom and edge all joints.
7. Weepholes not to be placed in wing.
8. If curbing is being removed to install a driveway and the gutter should become separated from the driving surface in excess of 1/16 inch, then the gutter shall also be removed and replaced.
9. Wings of the commercial driveway which are a portion of the sidewalk shall not exceed 8.333% (1:12).
10. ODOT Standard Drawings for driveways may be used when preapproved by City Engineer.
11. Slope of the driveway may be away from the curb when preapproved by City Engineer.

Beaverton Engineering	STANDARD COMMERCIAL DRIVEWAY	SCALE: NONE DATE: JUNE 2018	210

A3 STANDARD COMMERCIAL DRIVEWAY
SCALE: NTS

FROELICH ENGINEERS
17700 SW UPPER BOONES FERRY RD
PORTLAND, OREGON 97223
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Washington County
Support Services
Facilities and Parks Services Division
Capital Improvement Projects

169 N 1st Ave
Hillsboro, OR. 97124

503-846-8751
MFacilBeeExternal@co.washington.or.us
www.co.washington.or.us

REGISTERED PROFESSIONAL ENGINEER — OREGON — PRELIMINARY NOT FOR CONSTRUCTION — EXPIRES: 12/31/2018 — RYAN M. OSEYKELBOSCH

RENEWS: 12/31/

ARCHITECT PROJECT NUMBER:
22-010

HOLST

123 NE 3RD AVE.
SUITE 310
PORTLAND, OR
97232

HOLSTARC.COM

COUNTY PROJECT NUMBER:
2040

COUNTY PROJECT MANAGER:
STUART SPAFFORD

BUILDING NUMBER:
0371A

BUILDING NAME:
EVERGREEN (EVGR) - INTENSIVE SERVICES

DEPARTMENT NAME:
HEALTH AND HUMAN SERVICES

PROJECT NAME:
CATT - INTENSIVE SERVICES BUILDING (ISB)

PROJECT ADDRESS:
17911 NW EVERGREEN PLACE BEAVERTON, OR 97006

REVISION / ISSUE SCHEDULE

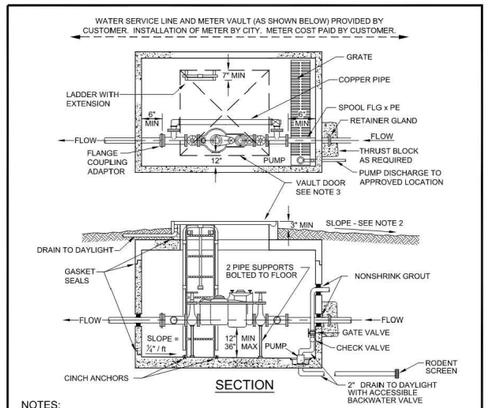
NO.	DESCRIPTION	DATE
A	SCHEMATIC DESIGN	11/16/22
B	50% DESIGN DEVELOPMENT	02/24/23
C	100% DESIGN DEVELOPMENT	04/21/23
D	DESIGN REVIEW	05/25/23
E	DESIGN REVIEW R1	09/01/23

SHEET NUMBER:
C-500

SHEET NAME:
DETAILS

DRAWN BY: **BLU**
SHEET ISSUE DATE: **04/19/23**
CHECKED BY: **EME**

STATUS:
DESIGN REVIEW



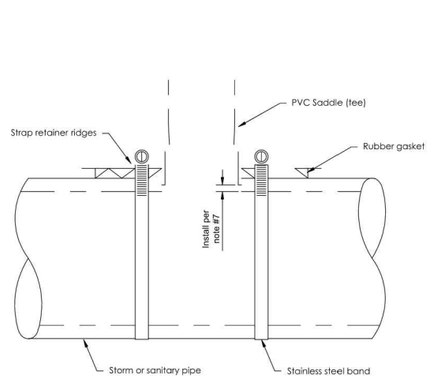
NOTES:
1. This is a public facility, governed by *UPC, SOPC* and *Oregon Dept. of Human Services*,as applicable.
2. Finished grade shall slope away from the vault cover to prevent ponding around the cover.
3. Vault door mechanisms shall not project below the ceiling of the vault interior.
4. Aluminum ladder with extension to be *O.S.H.A.* approved, (*Utility Vault Co.* 1672 or equal). Ladder to have clear access from vault door.
5. Seal pipe penetrations with nonshrink grout to achieve a water tight seal. Piping shall not be directly imbedded in concrete or masonry. Provide pipe straps for thrust resistance per *State of Oregon Plumbing Code (SOPC)*.
6. Check valve to be union style (compression) or install union with check valve.
7. Slope interior toward end of floor for sump or gravity drain outlet. Trim floor drain grating as required for pump installation. For alternate floor center drain, see *Drawing 690-4A*.

METER SIZE	UTILITY VAULT CO. VAULT MODEL NO.	BILCO - VAULT DOOR MODEL NO.
3"	676-WA	J-5AL
4" & 6"	687-WA	J-5AL
8"	810-LA	JD-3AL

Beaverton Engineering	COMPOUND WATER METER VAULT	SCALE: NONE	DATE: JUNE 2018	670-3

FILENAME: 670-3.dwg

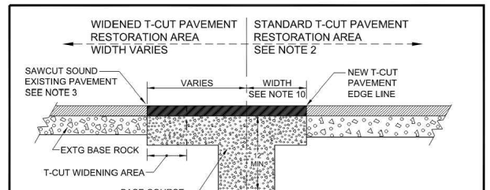
C1 3" COMPOUND WATER METER VAULT
SCALE: NTS



1. Indexed PVC gasketed saddle shall conform to ASTM 3034 SDR35.
2. PVC compounds shall conform to ASTM d1784 with cell class of 12454-B/C or 12364-C.
3. All elastomeric seals (rubber gaskets) shall conform to ASTM F477.
4. Indexed PVC gasketed saddle shall be installed with rubber gaskets approved for the sewer main wall configuration (profile or smooth wall).
5. Stainless steel bands shall be 300 series, ⅞" band width, cadmium plated, carbon steel, and attached with hex head, slotted screws.
6. Insertion hole shall be core drilled.
7. Indexed saddle shall be inserted a minimum of ½" from outside wall of host pipe and shall not protrude beyond the inside wall of the host pipe.

Beaverton Engineering	PVC GASKETED SADDLE	SCALE: NONE	DATE: JUNE 2018	520

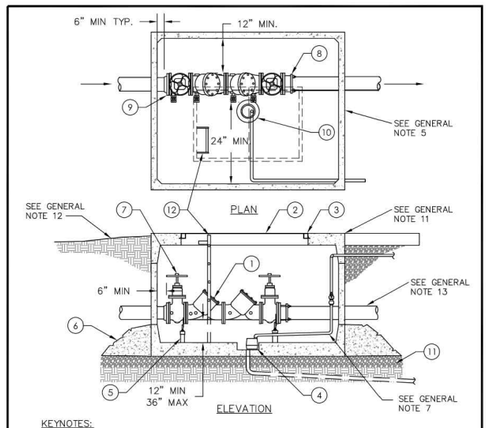
C2 PVC GASKETED SADDLE
SCALE: NTS



NOTES:
1. This drawing applies to trench cuts and other kinds of pavement cuts.
2. For width dimensions, see *Engineering Design Manual* and the notes that follow.
3. Sawcut existing AC pavement full depth. Sawcut existing PCC pavement according to Drawing 310
4. Restore existing pavement material(s) to pre-existing or better conditions. Thickness shall be as follows:
   a. For existing Portland Cement Concrete (PCC): existing pavement thickness plus 2 inches, but not less than 8 inches. On Arterial and Collector streets, concrete patching material shall be high early strength class 5,000 psi PCC approved by City Engineer.
   b. For existing Asphalt Concrete (AC): resurface to a minimum of 3½" of Level 2, ½" ACP or existing AC thickness plus 2 inches, whichever is greater, but do not exceed 6 inches. Compact AC in 2-inch maximum lifts to 91% of maximum density (*Rice*).
   c. Base course shall be 3/4"-0 aggregate compacted to 95% of max density as per *AASHTO* T-99 and *ODOT/APWA* Spec 00405. Minimum thickness shall be 8 inches under AC pavement and 2 inches under PCC pavement.
5. All cut edges of AC shall be sand sealed with CRS-1 or CRS-2 emulsified asphalt or equal.
6. If new edge of pavement is less than three feet from another patch, curb or edge of street, replace the pavement in between.
7. If any part of more than one pre-existing patch edge is within 3 feet of the T-cut, remove and restore the pavement to the far edge of the furthest pre-existing patch.
8. On longitudinal pavement cuts, the edge of the T-cut shall not lie in a wheel path. Width of T-cut shall be widened where necessary to move the edge out of the wheel path and either to parallel and 6 inches from the nearest lane line, or to the location required by note 6 or 7 above as applicable, whichever is the furthest from the pavement cut.
9. If an edge of a T-cut will be within 5 feet or less of an area of distressed existing pavement, the length and width of the pavement restoration shall be determined by City Engineer upon notification by the Contractor.
10. Width from edge of trench shall be 6 inches minimum for trenches that are 12 inches wide or less and 12 inches minimum for trenches wider than 12 inches. The width may be increased up to 10 feet on both sides of the pavement cut as approved by City Engineer.

Beaverton Engineering	PAVEMENT CUT RESTORATION	SCALE: NONE	DATE: JUNE 2018	302

C3 PAVEMENT CUT RESTORATION
SCALE: NTS

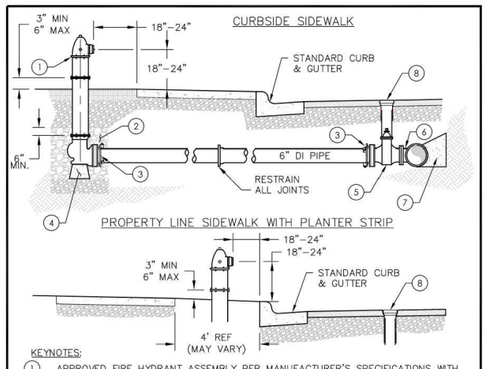


KEYNOTES:
(1) APPROVED DOUBLE CHECK (DC) ASSEMBLY PER STANDARDS
(2) CONCRETE VAULT WITH ACCESS HATCH DOORS
(3) TYPICAL VAULT DOOR
(4) SUMP PUMP INCLUDING 1-1/2" PVC DISCHARGE PIPING WITH CHECK VALVE, OR 4" PVC GRAVITY DRAIN WITH BACKWATER VALVE TO STORM SEWER. SEE BUILDING DEPT FOR REQUIRED PERMITS.
(5) TYPICAL PIPE SUPPORTS – SEE NOTE 10, DRAWING 690-4B
(6) TYPICAL CONCRETE BALLAST – SEE NOTE 8, DRAWING 690-4B
(7) TYPICAL NON-RISING STEM (N.R.S.) GATE VALVES
(8) FLANGE COUPLING ADAPTER
(9) ADAPTER FLANGE
(10) 12" DIA SUMP WITH PUMP OR GRAVITY DRAIN
(11) COMPACTED CLASS B BACKFILL BASE 6" MINIMUM
(12) OSHA-APPROVED LADDER – SEE DRAWING 670-1

Beaverton Engineering	3" AND LARGER DOUBLE CHECK VALVE ASSEMBLY (DC)	SCALE: NONE	DATE: JUNE 2018	690-4A

FILENAME: 690-4A.dwg

A1 3" DBL CHECK VALVE ASSEMBLY (DCVA)
SCALE: NTS



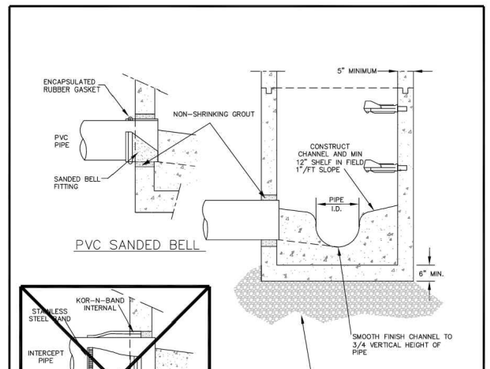
KEYNOTES:
(1) APPROVED FIRE HYDRANT ASSEMBLY PER MANUFACTURER'S SPECIFICATIONS WITH STORZ HPHA50-45NH PERMANENT HYDRANT ADAPTER ON THE 4½" PORT.
(2) DRAIN ROCK, 4 CU. FT. MINIMUM
(3) TYPICAL 6" MEGALUG RETAINER OR EQUAL AT EACH PIPE FITTING
(4) 12"x12"x8" CONCRETE PIER BLOCK PLACED ON UNDISTURBED NATIVE SOIL
(5) 6" MJxMJ GATE VALVE.
(6) WATER MAIN SIZE MJ x 6" MJ TAPPING SLEEVE, SEE DRAWING 630-2, SEE NOTE 4
(7) CONCRETE THRUST BLOCK, SEE DRAWING 630-9
(8) VALVE BOX, SEE DRAWING 640-2

NOTES:
1. REFER TO DRAWING 650-1 FOR HYDRANT CLEAR ZONE REQUIREMENTS
2. HYDRANT BURY DEPTH – 30" MIN., 6' MAX.
3. WHEN FINAL LIFT OF HMAC IS PLACED, CONTRACTOR TO SET A BLUE REFLECTOR BUTTON FOR THE FIRE DEPARTMENT. SEE DRAWING 650-4 FOR PLACEMENT
4. USE MJ OR FLG VALVE & TEE AS NEEDED TO PLACE VALVE BOX OUTSIDE OF WHEEL PATH.

Beaverton Engineering	FIRE HYDRANT ASSEMBLY INSTALLATION	SCALE: NONE	DATE: JUNE 2018	650-2

FILENAME: 650-2.dwg

A2 FIRE HYDRANT ASSEMBLY INSTALLATION
SCALE: NTS



NOTES:
1. ALL PRE-CAST MANHOLE SECTIONS SHALL CONFORM TO THE REQUIREMENTS OF ASTM C-478 AND DRAWING 330.
2. NON-FLEXIBLE PIPES SHALL HAVE A SHEAR JOINT WITHIN 18"-24" OF THE INSIDE WALL OF THE MANHOLE.

Beaverton Engineering	MANHOLE CONNECTIONS	SCALE: NONE	DATE: JUNE 2018	331

A3 MANHOLE CONNECTIONS
SCALE: NTS

FROELICH ENGINEERS
17700 SW UPPER BOONES FERRY RD
PORTLAND, OREGON 97223
(503) 624-7005
froelich-engineers.com

Washington County
Support Services
Facilities and Parks Services Division
Capital Improvement Projects
169 N 1st Ave
Hillsboro, OR. 97124
503-846-8751
MFacilBeeExternal@co.washington.or.us
www.co.washington.or.us

REGISTERED PROFESSIONAL ENGINEER — OREGON — JANUARY 9, 2018 — PEYAM A. TABRIZI — PRELIMINARY NOT FOR CONSTRUCTION
RENEWS: 12/31/

ARCHITECT PROJECT NUMBER:
22-010

HOLST
123 NE 3RD AVE.
SUITE 310
PORTLAND, OR
97232
HOLSTARC.COM

COUNTY PROJECT NUMBER:
2040

COUNTY PROJECT MANAGER:
STUART SPAFFORD

BUILDING NUMBER:
0371A

BUILDING NAME:
EVERGREEN (EVGR) - INTENSIVE SERVICES

DEPARTMENT NAME:
HEALTH AND HUMAN SERVICES

PROJECT NAME:
CATT - INTENSIVE SERVICES BUILDING (ISB)

PROJECT ADDRESS:
17911 NW EVERGREEN PLACE BEAVERTON, OR 97006

REVISION / ISSUE SCHEDULE

NO.	DESCRIPTION	DATE
A	SCHEMATIC DESIGN	11/16/22
B	50% DESIGN DEVELOPMENT	02/24/23
C	100% DESIGN DEVELOPMENT	04/21/23
D	DESIGN REVIEW	05/25/23
E	DESIGN REVIEW R1	09/01/23

SHEET NUMBER:
C-501

SHEET NAME:
DETAILS

DRAWN BY: **BLU**
SHEET ISSUE DATE: **04/19/23**
CHECKED BY: **EME**

STATUS:
DESIGN REVIEW

VAULT INFORMATION		
DCVA SIZE	OLDCASTLE VAULT NO.	DOOR MODEL NO.
4"	676-WA	676-T-2-332P
6"	687-WA	687-T-2-332P
8"	5106-WA	5106-3-T-2-332P
10"	5106-WA	5106-3-T-2-332P
OR APPROVED EQUAL		

GENERAL NOTES:

1. CONSULT WITH FIRE DEPARTMENT FOR SIZING OF DCDA.
2. THIS IS TO BE A PRIVATE FACILITY, GOVERNED BY OPSC, NFPA, OREGON HEALTH AUTHORITY, AND HILLSBORO FIRE MARSHAL, AS APPLICABLE.
3. ALL MJ JOINTS SHALL HAVE MECHANICAL JOINT RESTRAINTS.
4. CONTRACTOR TO SEAL ALL OPENINGS IN VAULT WITH NON-SHRINK GROUT OR "LINK-SEAL."
5. COAT ALL EXTERIOR VAULT SURFACES WITH CO-MA-SEAL, OR APPROVED EQUAL.
6. PROVIDE POWER SOURCE AS REQUIRED FOR SUMP PUMP. SECURE POWER CORD TO DISCHARGE PIPING WITH NYLON CABLE TIES. SEE DRAWING 670-2 FOR SUMP PUMP INSTALLATION DETAILS.
7. PLUMB PUMP DISCHARGE TO APPROVED LOCATION PER PLUMBING INSPECTOR.
8. IN AREAS PRONE TO HIGH GROUNDWATER POUR CONCRETE BALLAST (3 CUBIC YARDS MINIMUM); ENGINEER IS RESPONSIBLE TO ENSURE ADEQUATE BALLAST IS PROVIDED TO PREVENT FLOATING OF VAULT.
9. VAULT DOOR MECHANISMS SHALL NOT PROTRUDE BELOW THE CEILING OF THE VAULT INTERIOR.
10. ASSEMBLY IS TO BE SUPPORTED BY A SUBSTANTIAL RUST-RESISTANT PRODUCT SUCH AS "STANDON" OR APPROVED EQUAL TO PREVENT UNDUE STRESS OR STRAIN ON THE ASSEMBLY AND PIPING.
11. VAULT TOP SHALL BE SET LEVEL WITH ADJACENT HARD SURFACE (CONCRETE OR AC PAVEMENT).
12. FOR INSTALLATION IN LANDSCAPE AREA, PLACE VAULT TOP 3"-5" ABOVE GROUND SURFACE AND SLOPE GROUND AWAY AT ¼" PER FOOT TYPICAL. DO NOT BURY HATCH DRAIN.
13. EXTEND DUCTILE IRON PIPE 5' MINIMUM OUT OF VAULT TO PROTECT FROM BREAKING DUE TO VAULT SETTLEMENT.
14. SEE DRAWING 690-5A FOR PLAN AND ELEVATION VIEWS. TYPICAL LAYOUT SHOWN, MAY BE INSTALLED IN OPPOSITE CONFIGURATION TO CONFORM TO SITE CONDITION REQUIREMENTS.
15. DETECTOR METER SHALL BE MART & COMPATIBLE WITH SENSUS MX4.
16. WHEN LOCATED IN PEDESTRIAN WALKWAY A NON-SLIP LID AND GROUTED PICK HOLES ARE REQUIRED.

Beaverton ENGINEERING	3" AND LARGER DOUBLE CHECK DETECTOR ASSEMBLY (DETAIL NOTES)	SCALE: NONE	690-5C
		DATE: JUNE 2018	

FILENAME: 690-5C.dwg

C2 6" DCDA DETAIL NOTES

SCALE: NTS

VAULT INFORMATION		
DCVA SIZE	OLDCASTLE VAULT NO.	DOOR MODEL NO.
3"	577-LA	57-T-2-332P
4"	577-LA	57-T-2-332P
6"	676-WA	676-T-2-332P
8"	687-WA	687-T-2-332P
10"	5106-WA	5106-3-T-2-332P
OR APPROVED EQUAL		

GENERAL NOTES:

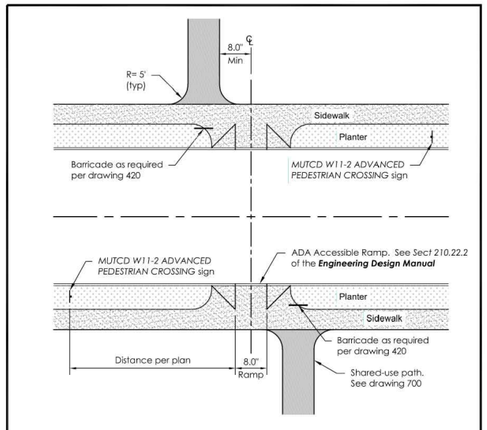
1. PLACE DC ASSEMBLY WITHIN PROPERTY LINE AS CLOSE TO METER AS POSSIBLE WITH NO CONNECTIONS OR TEES BETWEEN METER AND DC.
2. THIS IS TO BE A PRIVATE FACILITY, GOVERNED BY OPSC AND OREGON HEALTH AUTHORITY, AS APPLICABLE.
3. ALL MJ JOINTS SHALL HAVE MECHANICAL JOINT RESTRAINTS.
4. CONTRACTOR TO SEAL ALL OPENINGS IN VAULT WITH NON-SHRINK GROUT OR "LINK-SEAL."
5. COAT ALL EXTERIOR VAULT SURFACES WITH CO-MA-SEAL OR APPROVED EQUAL.
6. PROVIDE POWER SOURCE AS REQUIRED FOR SUMP PUMP. SECURE POWER CORD TO DISCHARGE PIPING WITH NYLON CABLE TIES. SEE DRAWING 670-2 FOR SUMP PUMP INSTALLATION DETAILS.
7. PUMP DISCHARGE TO APPROVED LOCATION PER PLUMBING INSPECTOR.
8. IN AREAS PRONE TO HIGH GROUNDWATER POUR CONCRETE BALLAST (3 CUBIC YARDS MINIMUM); ENGINEER IS RESPONSIBLE TO ENSURE ADEQUATE BALLAST IS PROVIDED TO PREVENT FLOATING OF VAULT.
9. VAULT DOOR MECHANISMS SHALL NOT PROTRUDE BELOW THE CEILING OF THE VAULT INTERIOR.
10. ASSEMBLY IS TO BE SUPPORTED BY A SUBSTANTIAL RUST-RESISTANT PRODUCT SUCH AS "STANDON" OR APPROVED EQUAL TO PREVENT UNDUE STRESS OR STRAIN ON THE ASSEMBLY AND PIPING.
11. VAULT TOP SHALL BE SET LEVEL WITH ADJACENT HARD SURFACE (CONCRETE OR AC PAVEMENT).
12. FOR INSTALLATION IN LANDSCAPE AREA, PLACE VAULT TOP 3"-5" ABOVE GROUND SURFACE AND SLOPE GROUND AWAY AT ¼" PER FOOT TYPICAL. DO NOT BURY HATCH DRAIN.
13. EXTEND DUCTILE IRON PIPE 5' MINIMUM OUT OF VAULT TO PROTECT FROM BREAKING DUE TO VAULT SETTLEMENT.
14. SEE DRAWING 690-4A FOR PLAN AND ELEVATION VIEWS.
15. CONSULT WITH BUILDING DEPARTMENT FOR PROPER SIZING OF DC.
16. WHEN LOCATED IN PEDESTRIAN WALKWAY A NON-SLIP LID AND GROUTED PICK HOLES ARE REQUIRED.

Beaverton ENGINEERING	3" AND LARGER DOUBLE CHECK VALVE ASSEMBLY (DETAIL NOTES)	SCALE: NONE	690-4B
		DATE: JUNE 2018	

FILENAME: 690-4B.dwg

C3 3" DCVA DETAIL NOTES

SCALE: NTS



NOTES:
Approved for use under the following conditions:

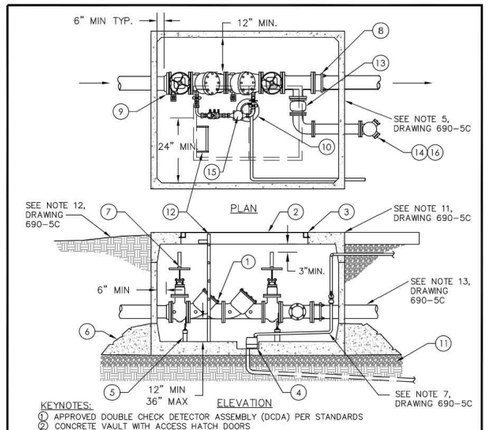
1. Locations per approved plan.
2. Neighborhood routes & local streets only.
3. Street has posted speed 25 mph or less.
4. Tangent sections or curves with 300' radius or larger
5. Street has no more than 2 travel lanes.

Other conditions require a design exception. See Sect 160 of the *Engineering Design Manual*.

Beaverton ENGINEERING	MID-BLOCK PEDESTRIAN CROSSING	SCALE: NONE	705
		DATE: JUNE 2018	

A2 MID-BLOCK PEDESTRIAN CROSSING

SCALE: NTS



KEYNOTES:

1. APPROVED DOUBLE CHECK DETECTOR ASSEMBLY (DCDA) PER STANDARDS
2. CONCRETE VAULT WITH ACCESS HATCH DOORS
3. TYPICAL VAULT DOOR
4. SUMP PUMP INCLUDING 1 ½" PVC DISCHARGE PIPING WITH CHECK VALVE, OR 4" PVC GRAVITY DRAIN WITH BACKWATER VALVE TO STORM SEWER
5. TYPICAL PIPE SUPPORTS, SEE NOTE 10, DRAWING 690-5C
6. TYPICAL CONCRETE BALLAST – SEE NOTE 8, DRAWING 690-5C
7. TYPICAL OS&Y GATE VALVES FOR DCDA, 3" MINIMUM CLEARANCE IN OPEN POSITION
8. FLANGE COUPLING ADAPTER
9. ADAPTER FLANGE
10. 12" DIA SUMP WITH PUMP OR GRAVITY DRAIN
11. COMPACTED CLASS B BACKFILL BASE 6" MINIMUM
12. OSHA-APPROVED LADDER – SEE DRAWING 670-1
13. CHECK VALVE WITH BALL DRIP VALVE PER NFPA 13 AND NFPA 24 STANDARDS
14. F.D.C. PER OREGON FIRE CODE, OREGON STRUCTURAL SPECIALTY CODE, NFPA 13, 13R, 14 & 24 STANDARDS. LOCATION AS APPROVED BY THE FIRE CODE OFFICIAL.
15. DETECTOR METER – SEE NOTE 15, DRAWING 690-5C
16. F.D.C. SHALL BE PROVIDED WITH LOCKING FDC PLUGS.

Beaverton ENGINEERING	3" AND LARGER DOUBLE CHECK DETECTOR ASSEMBLY	SCALE: NONE	690-5A
		DATE: JUNE 2018	

FILENAME: 690-5A.dwg

A3 6" DBL CHECK DETECTOR ASM (DCDA)

SCALE: NTS

FROELICH ENGINEERS
17700 SW UPPER BOONES FERRY RD
PORTLAND, OREGON 97223
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Washington County
Support Services
Facilities and Parks Services Division
Capital Improvement Projects

169 N 1st Ave
Hillsboro, OR. 97124

503-846-8751
MFacilBeeExternal@co.washington.or.us
www.co.washington.or.us

REGISTERED PROFESSIONAL ENGINEER OREGON JULY 10, 2018 PEYAN MEYKELBOSCH
PRELIMINARY NOT FOR CONSTRUCTION
RENEWS: 12/31/

ARCHITECT PROJECT NUMBER:
22-010

HOLST

123 NE 3RD AVE.
SUITE 310
PORTLAND, OR
97232

HOLSTARC.COM

COUNTY PROJECT NUMBER:
2040

COUNTY PROJECT MANAGER:
STUART SPAFFORD

BUILDING NUMBER:
0371A

BUILDING NAME:
EVERGREEN (EVGR) - INTENSIVE SERVICES

DEPARTMENT NAME:
HEALTH AND HUMAN SERVICES

PROJECT NAME:
CATT - INTENSIVE SERVICES BUILDING (ISB)

PROJECT ADDRESS:
17911 NW EVERGREEN PLACE BEAVERTON, OR 97006

REVISION / ISSUE SCHEDULE

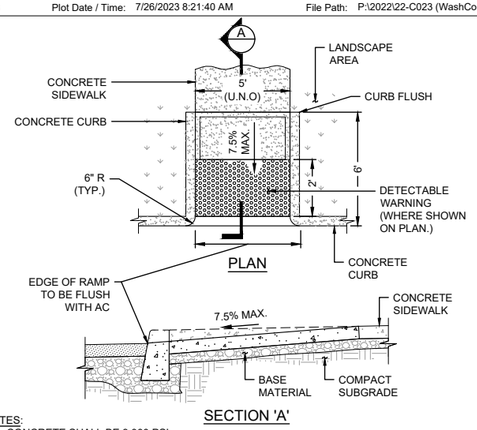
NO.	DESCRIPTION	DATE
A	SCHEMATIC DESIGN	11/16/22
B	50% DESIGN DEVELOPMENT	02/24/23
C	100% DESIGN DEVELOPMENT	04/21/23
D	DESIGN REVIEW	05/25/23
E	DESIGN REVIEW R1	09/01/23

SHEET NUMBER:
C-502

SHEET NAME:
DETAILS

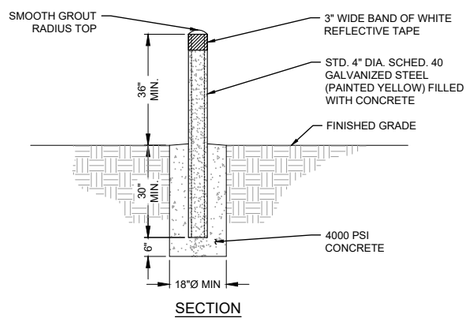
DRAWN BY: **BLU**
SHEET ISSUE DATE: **04/19/23**
CHECKED BY: **EME**

STATUS:
DESIGN REVIEW

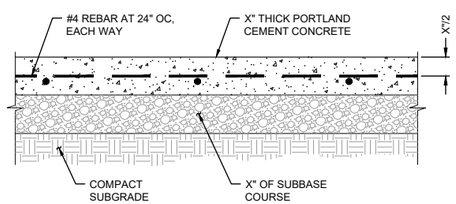


NOTES:
1. CONCRETE SHALL BE 3,000 PSI.

C1 CURB RAMP - TYPE 2
SCALE: NTS

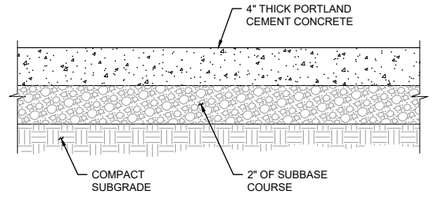


B1 PIPE BOLLARD (4" DIA)
SCALE: NTS



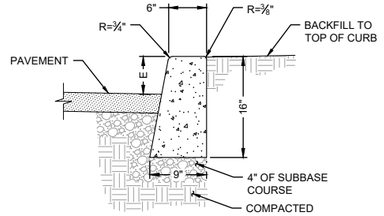
NOTES:
1. JOINTS:
   - CONSTRUCT CONTRACTION JOINTS AT 15' MAX. SPACING AND AT RAMPS.
   - CONSTRUCT EXPANSION JOINTS AT 200' MAX. SPACING AT POINTS OF TANGENCY AND AT ENDS OF EACH DRIVEWAY.
2. PROVIDE MEDIUM TO COARSE BROOM FINISH.

A1 HEAVY CONCRETE PAVEMENT SECTION
SCALE: NTS



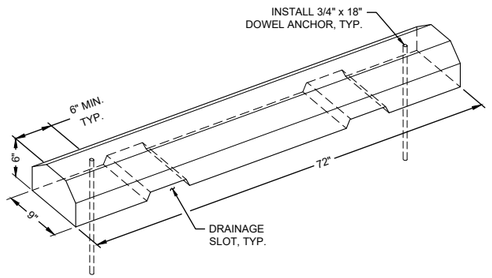
NOTES:
1. - CONSTRUCT CONTRACTION JOINTS AT 15' MAX. SPACING AND AT RAMPS.
   - CONSTRUCT EXPANSION JOINTS AT 200' MAX. SPACING AT POINTS OF TANGENCY AND AT ENDS OF EACH DRIVEWAY.
2. PROVIDE MEDIUM TO COARSE BROOM FINISH.

D2 CONCRETE PAVEMENT SECTION
SCALE: NTS



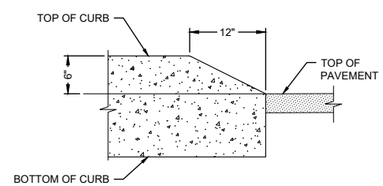
NOTES:
1. CURB EXPOSURE 'E' = 6", TYP. VARY AS SHOWN ON PLANS OR AS DIRECTED.
2. CONSTRUCT CONTRACTION JOINTS AT 15' MAX. SPACING AND AT RAMPS. CONSTRUCT EXPANSION JOINTS AT 200' MAX SPACING AT POINTS OF TANGENCY AND AT ENDS OF EACH DRIVEWAY.
3. TOPS OF ALL CURBS SHALL SLOPE TOWARD THE ROADWAY AT 2% UNLESS OTHERWISE SHOWN OR AS DIRECTED.
4. DIMENSIONS ARE NOMINAL AND MAY VARY TO CONFORM WITH CURB MACHINE AS APPROVED BY THE ENGINEER.

C2 STANDARD CONCRETE CURB
SCALE: NTS

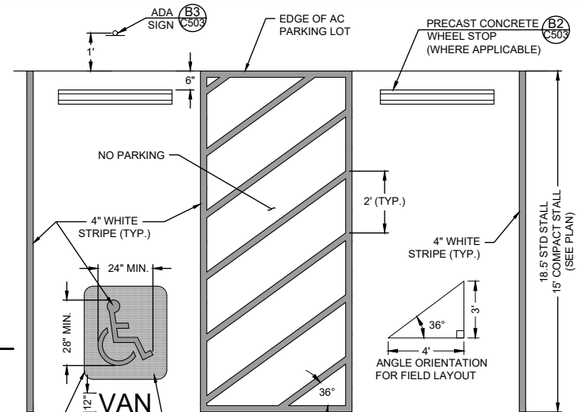


NOTES:
1. DIMENSIONS ARE NOMINAL AND MAY VARY TO CONFORM TO MANUFACTURER'S PRODUCTS APPROVED BY ENGINEER.

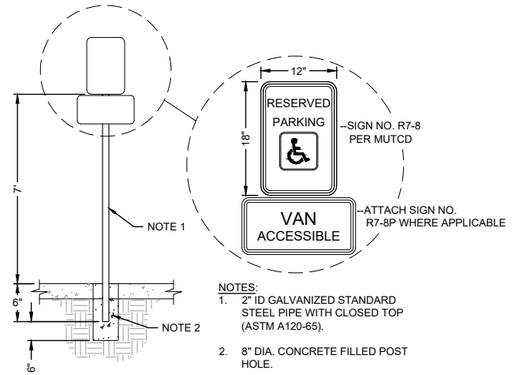
B2 PRECAST CONCRETE WHEEL STOP
SCALE: NTS



A2 CONCRETE CURB ENDING
SCALE: NTS

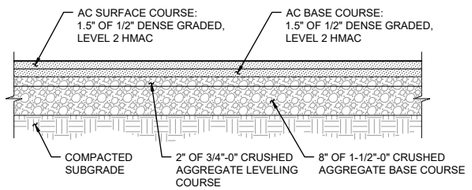


C3 TYPICAL PARKING LAYOUT
SCALE: NTS



NOTES:
1. 2" ID GALVANIZED STANDARD STEEL PIPE WITH CLOSED TOP (ASTM A120-65).
2. 8" DIA. CONCRETE FILLED POST HOLE.

B3 ADA PARKING SIGN - TYPE 1
SCALE: NTS



A3 ASPHALT PAVEMENT SECTION (ONSITE)
SCALE: NTS

Washington County
Support Services
Facilities and Parks Services Division
Capital Improvement Projects

169 N 1st Ave
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503-846-8751
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REGISTERED PROFESSIONAL ENGINEER
OREGON
JULY 10, 2018
ERIC M. EYKELBOSCH
PRELIMINARY NOT FOR CONSTRUCTION
RENEWS: 12/31/

FROELICH ENGINEERS
17700 SW UPPER BOONES FERRY RD
PORTLAND, OREGON 97223
(503) 624-7005
froelich-engineers.com

ARCHITECT PROJECT NUMBER:
22-010

HOLST
123 NE 3RD AVE.
SUITE 310
PORTLAND, OR
97232
HOLSTARC.COM

COUNTY PROJECT NUMBER:
2040

COUNTY PROJECT MANAGER:
STUART SPAFFORD

BUILDING NUMBER:
0371A

BUILDING NAME:
EVERGREEN (EVGR) - INTENSIVE SERVICES

DEPARTMENT NAME:
HEALTH AND HUMAN SERVICES

PROJECT NAME:
CATT - INTENSIVE SERVICES BUILDING (ISB)

PROJECT ADDRESS:
17911 NW EVERGREEN PLACE
BEAVERTON, OR 97006

REVISION / ISSUE SCHEDULE

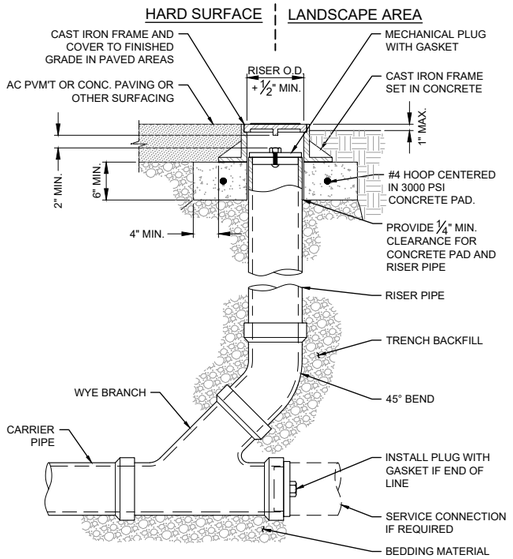
NO.	DESCRIPTION	DATE
A	SCHEMATIC DESIGN	11/18/22
B	50% DESIGN DEVELOPMENT	02/24/23
C	100% DESIGN DEVELOPMENT	04/21/23
D	DESIGN REVIEW	05/25/23
E	DESIGN REVIEW R1	09/01/23

SHEET NUMBER:
C-503

SHEET NAME:
DETAILS

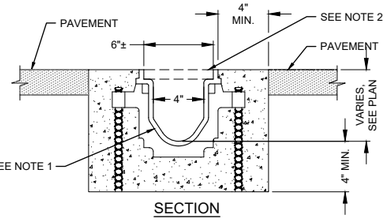
DRAWN BY: **BLU**
SHEET ISSUE DATE: **04/19/23**
CHECKED BY: **EME**

STATUS:
DESIGN REVIEW



NOTES:
1. CAST IRON FRAME AND COVER SHALL MEET H-20 LOAD REQUIREMENT.
2. FOR CARRIER PIPE SIZE 6"Ø AND LESS, PROVIDE RISER PIPE SIZE TO MATCH CARRIER PIPE.
3. FOR CARRIER PIPE SIZE 8"Ø AND LARGER, RISER PIPE SHALL BE 6"Ø.
4. RISER PIPE MATERIAL TO MATCH CARRIER PIPE MATERIAL.

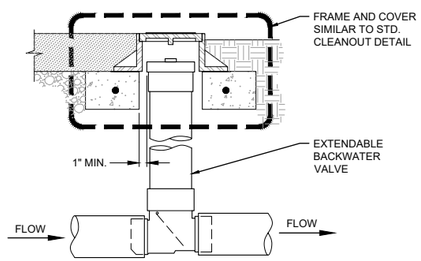
C1 STANDARD CLEANOUT (COTG)
SCALE: NTS



SECTION

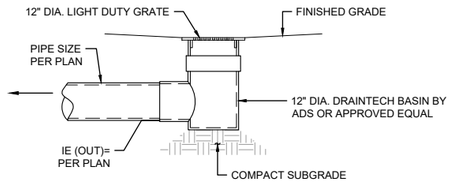
NOTES:
1. TRENCH DRAIN SHALL BE NDS DURA SLOPE TRENCH DRAIN. MINIMUM DEPTH = 6"
2. TRENCH DRAINS GRATE SHALL BE IRON AGE RAIN 6" HEEL PROOF RAIN TRENCH GRATE.
3. TRENCH SYSTEM SHALL BE INSTALLED PER MANUFACTURER'S INSTRUCTIONS.

B1 TRENCH DRAIN - 6 INCH WIDE
SCALE: NTS



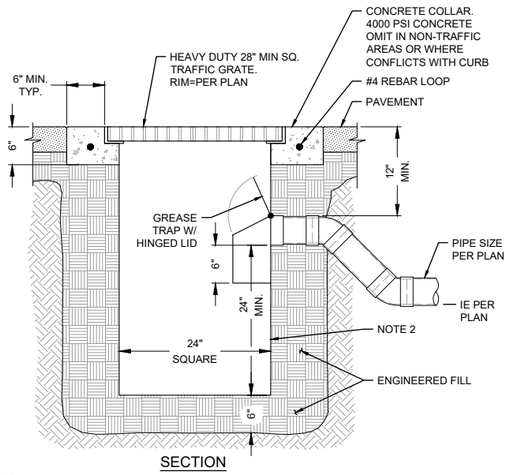
NOTES:
1. EXTENDABLE BACKWATER VALVE TO BE MANUFACTURED BY CLEAN CHECK OR APPROVED EQUAL AND SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.

A1 EXTENDABLE BACKWATER VALVE
SCALE: NTS



NOTES:
1. DRAIN AND GRATE BY ADS OR APPROVED EQUAL.

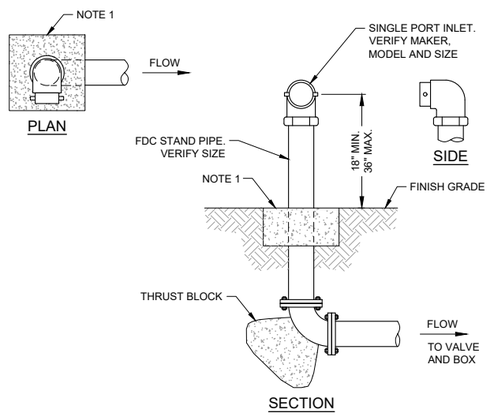
D2 AREA DRAIN - TYPE 1
SCALE: NTS



SECTION

NOTES:
1. CONTRACTOR TO WIDEN EXCAVATION AS REQUIRED TO OBTAIN COMPACTION WITH CONTRACTORS COMPACTION EQUIPMENT.
2. 1/4" STEEL PLATE, BITUMINOUS COATED. AS MANUFACTURED BY GIBSON STEEL BASINS OR APPROVED EQUAL.

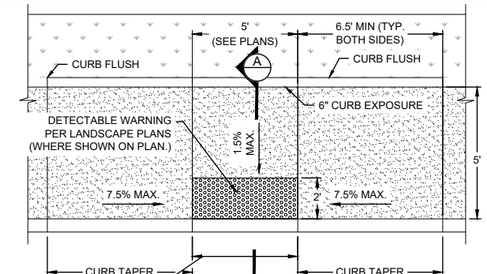
B2 TRAPPED CATCH BASIN
SCALE: NTS



SECTION

NOTES:
1. MIN. 3000 PSI CONCRETE ANCHOR PAD TO BE 12"x12"x6" THICK, UNLESS NOTED OTHERWISE. ELIMINATE IF INSTALLED IN CONCRETE PAVED AREA.
2. USE FLANGE OR THREADED FITTINGS.
3. CONTRACTOR SHALL PROVIDE SINGLE CHECK VALVE AND BALL DRIP VALVE IN ACCESSIBLE LOCATION INSIDE DCDA VAULT. COORDINATE WITH PLUMBING.

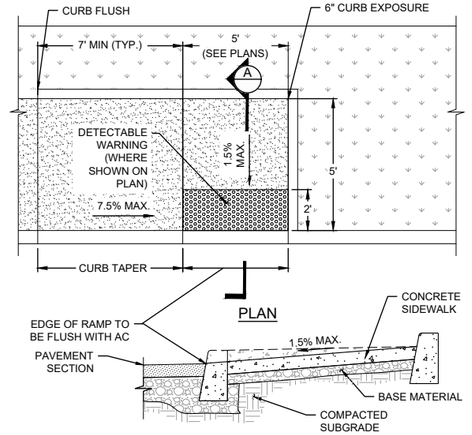
A2 FIRE DEPARTMENT CONNECTION (FDC) SINGLE PORT
SCALE: NTS



SECTION 'A'

NOTES:
1. CONCRETE SHALL BE 3,000 PSI.

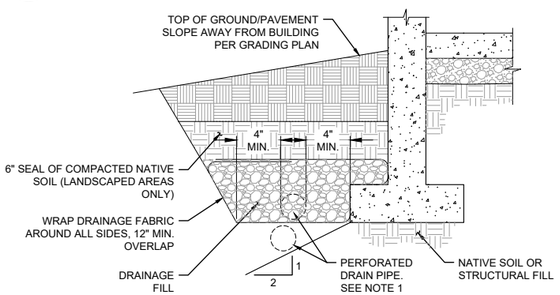
C3 CURB RAMP - TYPE 1
SCALE: NTS



SECTION 'A'

NOTES:
1. CONCRETE SHALL BE 3,000 PSI.

B3 CURB RAMP - TYPE 3
SCALE: NTS



NOTES:
1. LAY PERFORATED DRAIN PIPE ON MIN. 0.5% GRADIENT, WIDENING EXCAVATION AS REQUIRED. MAINTAIN PIPE ABOVE 2:1 SLOPE AS SHOWN.
2. CONNECT TO FOUNDATION DRAIN STUBOUT SHOWN ON PLANS.

A3 PERIMETER FOUNDATION DRAIN
SCALE: NTS

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SHEET NUMBER:
C-504

SHEET NAME:
DETAILS

DRAWN BY: **BLU**
SHEET ISSUE DATE: **04/19/23**
CHECKED BY: **EME**

STATUS:
DESIGN REVIEW

Appendix G: Operation and Maintenance

PROVIDED IN FINAL REPORT